



Stormwater Management Report

**under the
Massachusetts Stormwater
Management Regulations and Town
of Hamilton Rules & Regulations
Governing the Subdivision of Land**

3-Lot Residential Subdivision

**Assessor's
Map 31, Parcel 8**

**10 Meyer Lane
Hamilton, MA**

September 21, 2026

Applicant:

North Shore Property Group

Submitted to:

Town of Hamilton
Planning Board & Conservation Commission

Prepared by:



Griffin Engineering Group, LLC
Beverly, MA

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**STORMWATER
MANAGEMENT
CHECKLIST**



Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

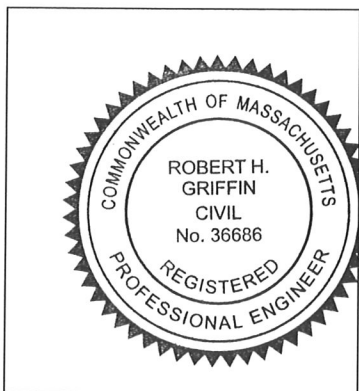
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



Robert H. Griffin

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- ☐ New development
- ☐ Redevelopment
- ☒ Mix of New Development and Redevelopment



Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- ☒ No disturbance to any Wetland Resource Areas
- ☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- ☐ Reduced Impervious Area (Redevelopment Only)
- ☒ Minimizing disturbance to existing trees and shrubs
- ☐ LID Site Design Credit Requested:
 - ☐ Credit 1
 - ☐ Credit 2
 - ☐ Credit 3
- ☐ Use of "country drainage" versus curb and gutter conveyance and pipe
- ☐ Bioretention Cells (includes Rain Gardens)
- ☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- ☐ Treebox Filter
- ☐ Water Quality Swale
- ☐ Grass Channel
- ☐ Green Roof
- ☐ Other (describe): _____

Standard 1: No New Untreated Discharges

- ☒ No new untreated discharges
- ☐ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- ☐ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

- ☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- ☒ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- ☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- ☒ Soil Analysis provided.
- ☒ Required Recharge Volume calculation provided.
- ☐ Required Recharge volume reduced through use of the LID site Design Credits.
- ☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - ☒ Static
 - ☐ Simple Dynamic
 - ☐ Dynamic Field¹
- ☐ Runoff from all impervious areas at the site discharging to the infiltration BMP.
- ☒ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- ☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- ☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
 - ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
 - ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- ☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- ☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- ☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- ☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- ☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - ☒ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - ☐ is within the Zone II or Interim Wellhead Protection Area
 - ☐ is near or to other critical areas
 - ☒ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - ☐ involves runoff from land uses with higher potential pollutant loads.
 - ☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - ☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

- ☐ The BMP is sized (and calculations provided) based on:
 - ☒ The ½" or 1" Water Quality Volume or
 - ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- ☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- ☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- ☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- ☐ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- ☐ The NPDES Multi-Sector General Permit does **not** cover the land use.
- ☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- ☐ All exposure has been eliminated.
- ☐ All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- ☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- ☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- ☐ Critical areas and BMPs are identified in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- ☐ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
 - ☐ Limited Project
 - ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - ☐ Bike Path and/or Foot Path
 - ☐ Redevelopment Project
 - ☒ Redevelopment portion of mix of new and redevelopment.
- ☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- ☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- ☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- ☐ The project is **not** covered by a NPDES Construction General Permit.
- ☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- ☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- ☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - ☒ Name of the stormwater management system owners;
 - ☒ Party responsible for operation and maintenance;
 - ☒ Schedule for implementation of routine and non-routine maintenance tasks;
 - ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
 - ☒ Description and delineation of public safety features;
 - ☒ Estimated operation and maintenance budget; and
 - ☒ Operation and Maintenance Log Form.
- ☐ The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- ☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- ☒ An Illicit Discharge Compliance Statement is attached;
- ☐ NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

ATTACHMENT A

PROJECT DESCRIPTION

1.0 INTRODUCTION

This stormwater management report was prepared in support of the proposed residential subdivision at 10 Meyer Lane (Assessors Map 31, Lot 8) in Hamilton, MA. The project consists of constructing 460-ft of new road and creating three single-family lots (one existing and two proposed).

According to the Massachusetts Department of Environmental Protection (MassDEP) Stormwater Management regulations, the proposed construction is exempt from MassDEP Stormwater Management requirements since the proposed development is a small residential subdivision (four or fewer lots). However, the regulations establish a framework for evaluating stormwater management systems that is used by the Town of Hamilton in evaluating new development projects. The narrative below compares the proposed construction to the MassDEP Stormwater Management requirements.

1.1 Existing Conditions

The 9.95-acre property is currently developed with a single-family residence with an attached garage, barn, storage shed, brick patio, pool with concrete patio, paved and gravel driveways, and landscaping. The parcel contains two wetland resource areas. Wetland A is a BVW located on the north side of the site and is part of a large wetland resource area that is approximately 63-acres in size. Wetland B is an approximately 2.17-acre BVW with upland pockets located centrally on the parcel. The property is bounded to the east and west by residential properties, north by a large wetland resource area, and to the south by Meyer Lane.

Topography across the site is moderately sloped terrain and varies from approximately elevation 97-ft (NAVD88 Datum) on the southeast side of the site near Meyer Lane to 40-ft (NAVD88 Datum) on the north side of the site adjacent to Wetland A. Most of the site drains to the large wetland resource area. The north side (approximately 61% of the parcel) slopes directly towards Wetland A and the south side of the site (approximately 37% of the parcel) slopes towards Wetland B which slopes west towards 100 Meyer Road. This stormwater runoff flows west across 100 Meyer Lane and eventually drains to the large wetland resource area. A small portion of the site (approximately 2%) drains toward Meyer Road.

1.2 Proposed Conditions

The project involves constructing a new 460-ft long roadway with a NFPA hammerhead turnaround to serve two new single-family residences with associated driveways, utilities, septic systems, stormwater management features, and landscaping. The existing approximately 14-ft wide roadway located on the subject property along the site frontage will be relocated to be within the 50-ft wide Meyer Lane private right-of-way and widened to 20-ft. Site plans showing the proposed development and stormwater management features are separately provided.

The project is considered a mixture of new development and redevelopment under the

Stormwater Management regulations. When complete, the site will have approximately 31,611 square feet of impervious surfaces, consisting of the roadway, driveways, and three residences. This is an approximately 16,671 square feet increase in impervious surface as compared to current conditions.

2.0 STORMWATER MANAGEMENT STANDARDS

2.1 Standard 1: No New Untreated Discharges

No new untreated discharges to wetlands or waterways proposed.

2.2 Standard 2: Peak Rate Attenuation

Hydrologic modeling was conducted using the HydroCAD computer model. This model uses an approximation of Soil Conservation Service TR-20 methods to calculate runoff rates and volumes based on descriptions of land use, ground characteristics, and size.

The time of concentration (T_c) for each subcatchment was calculated in HydroCAD using a combination of sheet flow and shallow concentrated flow. Sheet flow uses roughness coefficients (Manning's n) and watercourse slope to calculate travel time of stormwater runoff for each subcatchment. The site was modeled using a maximum of 100-feet of sheet flow. The shallow concentrated flow method was used to determine the velocity factor along the flow path of the runoff and thereby derive a travel time. The time of concentration of each subcatchment is the combination of these travel times. A minimum time of concentration of 6 minutes was used for all subcatchments, for both existing and proposed conditions.

Post-development peak runoff rates do not exceed pre-development peak runoff rates at all locations for the 2, 10, 25, and 100-year, 24-hour design storm events except for a minor increase towards Meyer Road. The area tributary to Meyer Road is small (6,915 sq. ft.) and can be considered "de minimus" under Massachusetts Stormwater Management Handbook criteria. Comparison of pre- and post-development stormwater runoff calculation results are summarized below in Table 1 to 4. (the HydroCAD drainage calculations for the aforementioned storm events are provided in Attachment F).

Table 1: Comparison of Pre-Development and Post-Development Peak Runoff Rates Towards 100 & 104 Meyer Road ⁽¹⁾ (Reach D1 in HydroCAD Model)

<u>Subcatchment</u>	Storm Frequency			
	2-Year	10-Year	25-Year	100-Year
Pre-Development	2.95	6.02	8.17	10.88
Post-Development	2.90	5.86	8.12	10.85

1. Flow rates measured in cubic feet per second (cfs).

Table 2: Comparison of Pre-Development and Post-Development Peak Runoff Rates Towards Large Wetland System ⁽¹⁾ (Reach D2 in HydroCAD Model)

<u>Subcatchment</u>	Storm Frequency			
	2-Year	10-Year	25-Year	100-Year
Pre-Development	3.80	8.57	11.99	16.39
Post-Development	3.65	8.11	11.67	15.87

1. Flow rates measured in cubic feet per second (cfs).

Table 3: Comparison of Pre-Development and Post-Development Peak Runoff Rates Towards 30 Boardman Lane ⁽¹⁾ (Reach D3 in HydroCAD Model)

<u>Subcatchment</u>	Storm Frequency			
	2-Year	10-Year	25-Year	100-Year
Pre-Development	0.15	0.32	0.44	0.60
Post-Development	0.15	0.32	0.44	0.60

1. Flow rates measured in cubic feet per second (cfs).

Table 4: Comparison of Pre-Development and Post-Development Peak Runoff Rates Towards Meyer Road ⁽¹⁾ (Reach D4 in HydroCAD Model)

<u>Subcatchment</u>	Storm Frequency			
	2-Year	10-Year	25-Year	100-Year
Pre-Development	0.29	0.56	0.75	0.97
Post-Development	0.38	0.61	0.76	0.94

1. Flow rates measured in cubic feet per second (cfs).

2. De Minimus increase. See narrative.

The volume of stormwater runoff leaving the site decreases approximately 5% during the 100-year storm event. Therefore, there is no threat of increased off-site flooding as a result of the proposed project.

2.3 Standard 3: Recharge

Site soils are mapped by the United States Department of Agriculture – Soil Conservation Service (SCS) (Essex Co. Massachusetts Southern Part Soil Survey, 1984) as being primarily Merrimac fine sandy loam (Hydrologic Soil Group 'B') on the west and north sides of the site and Woodbridge fine sandy loam (Hydrologic Soil Group 'C/D') on the east and south sides of the site. Six soil tests were performed at the site generally confirmed loamy fine sand. Wetland B, located centrally on-site, is mostly likely comprised of Woodbridge soils. Hydrologic Soil Group 'B' (HSG) was assigned to site soils for the recharge volume calculations and for the HydroCAD runoff calculations. Estimated seasonal high groundwater table was estimated to be approximately 94-inches below grade based on test pit observations.

For B-type soils, the Stormwater Management guidelines require that a recharge device be provided with a capacity equal to 0.35-inches times the new impervious area. For this project, the minimum design groundwater recharge volume required is 486 cf. Since more than 100% of the new impervious surfaces are tributary to the proposed stormwater recharge device, the required recharge volume was not required to be adjusted. The provided recharge volume below the lowest outlet of the infiltration basin, six drywells, and three infiltration trenches is approximately 3,891 cf., which substantially exceeds the required volume. Calculations are provided in Attachment B.

As noted above, test pits were performed to analyze subsurface soil conditions and assign an exfiltration rate. The native soil is primarily characterized as loamy fine sand. Therefore, the applicable Rawls Rate used for the drawdown calculations of the stormwater infiltration devices is 2.41 inches per hour. At this rate, the infiltration structures will drawdown in less than 18 hours. This is in compliance with the maximum 72-hours allowed by MassDEP Stormwater Management Regulations. Drawdown calculations are provided in Attachment C.

2.4 Standard 4: Water Quality

Since the project is considered a mix of new development and redevelopment and has a rapid infiltration rate (>2.4 inches/hour), the minimum required water quality treatment volume for the project site is the first inch of runoff from the new impervious surfaces. For this project, the required water quality volume is 1,389 cf (16,671 sf x 1-inch). The infiltration basin provides approximately 2,107 cf of water quality volume. The reader is referred to Water Quality Volume Calculations provided in Attachment C.

The TSS removal requirement is satisfied by treatment trains. The new roadway runoff will be directed to catch basins equipped with 4-ft deep-sumps and hood, a proprietary treatment device (First Defense Structure), and an infiltration basin. This system is predicted to achieve an annual TSS removal rate of 97%. The reader is referred to the TSS Removal Calculation Worksheet in Attachment C.

Due to the rapid infiltration rate onsite, at least 44% TSS is required to be removed from the roadway stormwater runoff prior to discharging to an infiltration structure. This is achieved by the deep-sump-hooded catch basins and proprietary treatment device.

As required by the Stormwater Management Standards, a Long-Term Pollution Prevention Plan is provided in Attachment C. In short the plan identifies suitable measures for source control and pollution prevention.

2.5 Standard 5: Land Uses with Higher Potential Pollutant Loads

In accordance with the Stormwater Management Standards, the proposed daycare use is not considered a Land Use with Higher Potential Pollutant Loads. Therefore, this standard does not apply to this project.

2.6 Standard 6: Critical Areas

The project site is not tributary to an environmentally-critical area as defined by the Massachusetts Stormwater Management Standards. Therefore, this standard does not apply to this project.

2.7 Standard 7: Redevelopment and Other Projects Subject to the Standards only to the Maximum Extent Practicable

The site is developed with a single-family dwelling with attached garage and associated paved driveway, gravel driveway, walks, pool with patio, and barn. The 16,671 square feet increase in impervious area is considered new development. The remaining portion of the project site is redevelopment (14,940 sq. ft.).

The project meets all stormwater standards as they apply to this mix of new development and redevelopment project. The work will improve stormwater management at the site. The proposed project will decrease peak stormwater runoff rate for the 2-, 10-, 25- and 100-year storm events except for a small portion of the new road which drains towards Meyer Road; does not increase off-site flooding during 100-year storm event; provides the required recharge volume, water quality volume and 80% TSS removal rate for new impervious surfaces; and includes the required LTPPP, mitigation measures and Operation and Maintenance Plan.

The proposed relocation and widening of the pavement within the Meyer Lane right-of-way meets Standard 2 and the pretreatment portion of Standard 3 to the maximum extent practicable. Widening the roadway from approximately 14-ft wide to 20-ft increases impervious surface in this area by approximately 1,685 sf which causes the post-development peak runoff rate to be slightly higher than pre-development rates for the 2, 10, and 25-year storms. Since, there is no municipal drainage system within Meyer Road adjacent to Meyer Lane and the site, no pretreatment is provided similar to existing conditions. Due to the small size of this area and its location downstream of the project, the de minimus provision applies under the criteria listed in the Massachusetts Stormwater Management Handbook.

2.8 Standard 8: Construction Period Pollution Prevention and Erosion and Sediment Control

The NPDES Construction General Permit requires that a Stormwater Pollution Prevention Plan (SWPPP) be prepared for any project disturbing over 1-acre of land. Since this proposed project will disturb more than a 1-acre of land, a SWPPP is required. In accordance with the Massachusetts Stormwater Management Standards and the General Permit, a SWPPP will be prepared prior to land disturbance commencing.

The proposed project includes a comprehensive set of mitigation measures to protect the surrounding sites from impacts due to construction. Prior to work commencing on-site, there will be a preconstruction conference with the contractor. The purpose of this meeting will be to coordinate the best methods for erosion and sedimentation control and other construction-related issues. The implementation of a comprehensive soil and erosion control plan will occur prior to any construction activities within the project area. In general, the following sequence of events will occur:

- Erosion and sedimentation control devices will be installed along the edge of the down-gradient side of the project area prior to construction as depicted on the site plan. In addition to providing for sedimentation deposition and reducing runoff during storm events, this barrier will limit the work area.
- Erosion and sedimentation control devices will be inspected daily during periods of active construction and bi-weekly during the remainder of the construction period. Sediments will be removed from the barriers as soon as they reach a depth of 6-inches.
- Runoff from the site will be directed through sedimentation control barriers.
- During construction, disturbed areas will be kept to a minimum and vegetative stabilization of these areas will occur as soon as practicable.
- Temporary seeding, mulching, or other suitable stabilization measures will be used to protect exposed critical areas should unprotected soils remain exposed for prolonged periods.

Construction activities shall be monitored on-site by the construction supervisor to ensure that the soil erosion and sediment control features are installed properly, maintained, and to evaluate the need for additional erosion control and/or stabilization measures. The inspector will perform the following tasks:

- Supervise the installation and maintenance of the soil erosion and sediment control features.
- Evaluate the need for additional soil erosion and sediment control features.

- Scheduled inspections of erosion control features, including construction entrance, haybales, and dust control.
- Supervise and monitor temporary and permanent stabilization activities.

2.9 Standard 9: Operation and Maintenance Plan

An Operations & Maintenance plan has been provided (Attachment D). The owner of lot will be the party responsible for operation and maintenance of the proposed stormwater management devices.

2.10 Standard 10: Illicit Discharges

The submitted Long-Term Pollution Prevention Plan (Attachment C) specifies measures to prevent illicit discharges from entering the stormwater management system. Source control and response plans are also specified to prevent illicit discharges from being conveyed through the stormwater management system.

Consistent with the Massachusetts Stormwater Handbook, a signed Illicit Discharge Compliance Statement will be finalized prior to discharging stormwater to the post-construction stormwater BMP's. A draft copy of the Illicit Discharge Statement is provided (Attachment E).

3.0 SUMMARY

The proposed drainage system and site development plans for the three-lot residential subdivision conform to the MassDEP Stormwater Management Regulations. The proposed drainage system will treat and remove TSS and other pollutants throughout the project area and minimize erosion. Proper construction and operation and maintenance of the proposed drainage system are critical to its long-term performance. To that end, an Operations and Maintenance Plan and Long-Term Pollution Prevention Plan have been prepared and will be instituted.

ATTACHMENT B

STORMWATER COMPUTATIONS:

- **RECHARGE VOLUME CALCULATIONS**
- **WATER QUALITY VOLUME CALCULATIONS**
- **TSS REMOVAL CALCULATIONS**
- **DRAWDOWN CALCULATIONS**



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RECHARGE VOLUME & DESIGN CALCULATIONS

Job Name: 10 Meyer Ln, Hamilton

Job No: 2371

Date 9/21/2026

Designer MBP

Checked By: RHG

IMPERVIOUS AREA:

	Impervious	Total (sf)
Existing	14,940	14,940
Proposed	31,611	31,611
Net Increase		16,671

Target Depth Factor:

"B" Type Soils

0.35 inches of runoff

MINIMUM RECHARGE REQUIRED:

$$\text{Volume} = (16,671 \text{ sf}) \times (0.35 \text{ in}) \times (1' / 12 \text{ in}) = 486 \text{ cf}$$
$$\text{TOTAL} = 486 \text{ cf}$$

Infiltration Basin

Tributary Impervious Area = 13,082 sf
Minimum Volume Req'd = 382 cf
Storage Volume Provided = 2,107 cf **OK** (below lowest outlet)

Individual Lot Drywells (Two per Lot - 6 x 1,000 Gal)

Tributary Impervious Area = 8,400 sf
Minimum Volume Req'd = 245 cf
Storage Volume Provided = 1,254 cf **OK** (below lowest outlet)

Driveway Infiltration Trench (One per Lot)

Tributary Impervious Area = 4,554 sf
Minimum Volume Req'd = 245 cf
Storage Volume Provided = 530 cf **OK** (below lowest outlet)

Impervious Area Tributary to Infiltration BMPs: 21,482 sf
Percentage of Total Impervious Surfaces: 100.0% **OK**
Ratio of Total Impervious Area to Tributary Area: 1.00
Adjusted Min. Required Recharge Volume: 486 cf
Total Storage Provided: 3,891 cf **OK**



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495 Cabot Street, 2nd Floor
Beverly, MA 01915

Phone: 978-927-5111; Fax: 978-927-5103

WATER QUALITY VOLUME & DESIGN CALCULATIONS

Job Name: 10 Meyer Ln, Hamilton

Job No: 2371

Date 9/21/2026

Designer MBP

Checked By: RHG

REQUIRED WATER QUALITY VOLUME (WQv): 1 inch

IMPERVIOUS AREA INCREASE: 16,671 cf

MINIMUM WATER QUALITY VOLUME REQUIRED:

Volume = (16,671 sf) x (1 in) x (1'/12 in) = 1,389 cf

TOTAL = 1,389 cf

Infiltration Basin:

Tributary Impervious Area: 13,082 sf (ref HydroCAD calculations)

BMP Required WQV: 1,090 cf (13,082 sf * 1 in) / (12 in / ft)

Provided Storage Volume: 2,107 cf (below lowest outlet, ref HydroCAD calculations)

Driveway Infiltration Trenches:

Tributary Impervious Area: 4,554 sf (ref HydroCAD calculations)

BMP Required WQV: 380 cf (4,554 sf * 1 in) / (12 in / ft)

Provided Storage Volume: 530 cf (ref HydroCAD calculations)

2,107 cf + 530 cf > 1,389 cf **OK**

INSTRUCTIONS:

Non-automated: Mar. 4, 2008

- 1. Sheet is nonautomated. Print sheet and complete using hand calculations. Column A and B: See MassDEP Structural BMP Table
- 2. The calculations must be completed using the Column Headings specified in Chart and Not the Excel Column Headings
- 3. To complete Chart Column D, multiple Column B value within Row x Column C value within Row
- 4. To complete Chart Column E value, subtract Column D value within Row from Column C within Row
- 5. Total TSS Removal = Sum All Values in Column D

Location: Roadway (P6, P7, P8, P9) - Treatment Train 1

TSS Removal Calculation Worksheet					
A	B	C	D	E	
				TSS Removal Rate ¹	Starting TSS Load*
BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (B*C)	Remaining Load (C-D)	
Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75	
First Defense	0.80	0.75	0.60	0.15	
Infiltration Basin	0.80	0.15	0.12	0.03	

Separate Form Needs to be Completed for Each Outlet or BMP Train

97%

Total TSS Removal =

Project:	10 Meyer Ln, Hamilton
Prepared By:	
Date:	9/21/2026

*Equals remaining load from previous BMP (E) which enters the BMP

INSTRUCTIONS:

- 1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
- 2. Select BMP from Drop Down Menu
- 3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Location: Driveways (P11, P12, P13) - Treatment Train 2

B	C	D	E	F
BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
Infiltration Trench	0.80	1.00	0.80	0.20
Vegetated Filter Strip >50 feet	0.45	0.20	0.09	0.11
	0.00	0.11	0.00	0.11
	0.00	0.11	0.00	0.11
	0.00	0.11	0.00	0.11

Separate Form Needs to be Completed for Each Outlet or BMP Train

Total TSS Removal =

89%

Project: 10 Meyer Ln, Hamilton

Prepared By:

Date: 9/21/2026

*Equals remaining load from previous BMP (E) which enters the BMP



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Beverly, MA 01915

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DRAWDOWN ANALYSIS

Job Name: 10 Meyer Ln, Hamilton
Job No: 2371
Date 9/21/2026
Designer MBP
Checked By: RHG

DRAWDOWN CALCULATION:

Drawdown Time = $R_v / (K \cdot A)$ (hrs)

R_v = Storage Volume (cf) below lowest outlet

A = Bottom Area of Infiltration Field (sf)

K = Infiltration Rate (in/hr)

RECHARGE STRUCTURES

	R_v	A	K	Drawdown Time
Infil. Basin	3,874	1,113	2.41	17.3
1,000-Gal Drywell	209	95	2.41	11.0
Infil. Trench (Lot 1)	168	168	2.41	5.0
Infil. Trench (Lot 2)	140	140	2.41	5.0
Infil. Trench (Lot 3)	222	222	2.41	5.0

Drawdown is less than 72 hours.

Note:

-The infiltration rate used is the Rawls Rate for Loamy Fine Sand soils which were observed on-site in test pits conducted by Griffin Engineering Group.

ATTACHMENT C

**LONG-TERM POLLUTION
PREVENTION PLAN**

LONG-TERM POLLUTION PREVENTION PLAN

The Long-Term Pollution Prevention Plan (LTPPP) is incorporated into the long-term operation and maintenance plan of the projects stormwater management system. The purpose of the LTPPP is to identify potential sources of pollution that may affect the quality of stormwater discharges and describe suggested practices to reduce pollutants in stormwater discharges.

Good housekeeping practices - The property owners of the three lots are to keep the site in a neat and orderly condition so that pollutants are not conveyed to the stormwater management system or the bordering vegetated wetland. Materials swept, blown or washed into the storm drains can decrease the system's effectiveness. Some examples of good housekeeping practices are pavement sweeping, litter control, contained outdoor waste, cigarette disposal receptacles, and protected material storage areas.

Provisions for storing materials and waste products inside or under cover – There are no exterior (un-covered) storage areas associated with the project site.

Vehicle washing controls – Outdoor vehicle washing has the potential of conveying wash water with heavy concentrations of detergents and sediments into the stormwater drainage system. The project site does not include any designated vehicle washing areas, nor is it expected that frequent vehicle washing will take place on-site.

Requirements for routine inspections and maintenance of stormwater BMP's - Consistent with Standard 9 of the Massachusetts Stormwater Management Regulations, an Operation and Maintenance Plan has been provided in the Stormwater Management Report. The plan details routine inspection and maintenance of the stormwater BMP's along with associated record keeping forms.

Spill prevention and response plans – Sources of potential spill hazards include vehicle fluids and fuels, pesticides, paints, solvents, and liquid cleaning products. The majority of the spill hazards would likely occur within the buildings and would not enter the stormwater drainage system. However, there are spill hazards from vehicle fluids and fuels located outside of the buildings. These exterior spill hazards have the potential to enter the stormwater drainage system and are to be addressed as follows:

- 1) Spill hazards of pesticides, paints, and solvents shall be remediated using the Manufacturers' recommended spill cleanup protocol.
- 2) Vehicle fluid and fuel spills shall be remediated according to local and state regulations governing fuel spills.
- 3) The property owner shall have the following equipment and materials on hand to address a spill clean-up: brooms, dust-pans, mops, rags, gloves, trash bags, trash containers, and absorptive materials such as sand, sawdust, or kitty litter.

- 4) Spills of toxic or hazardous materials shall be reported to the Massachusetts Department of Environmental Protection at 1-888-304-1133.

Provisions for maintenance of lawns, garden, and other landscaped areas - It should be a general goal of the subject property owners to achieve a high-quality, well-groomed and stable landscape that evolves throughout the seasons and protects the overall condition of the property. All landscaped areas are to be maintained with dense vegetative growth or a layer of mulch so as to minimize sediment transport. Litter and waste are to be removed weekly from roadway, landscaped areas, lawn and driveway and disposed of properly.

Requirements for storage and use of fertilizer, herbicides, and pesticides - Fertilizers, herbicides, and pesticides are not to be stored on-site or within the buildings. Should use of some become necessary, application should be performed by a state licensed contractor in accordance with the manufacturer's label instruction and when environmental conditions are conducive to product application

Pet waste management provisions - All pet waste is to be scooped up, sealed in a plastic bag, and disposed of properly in the garbage. Pet waste should never be deposited in any stormwater management system or wetland for it contains high level of fecal coliform bacteria.

Provisions for operation and management of septic systems – The septic system will be operated and maintained in accordance with Title 5 and Town of Hamilton requirements.

Snow disposal and deicing chemicals – The individual property owners will be responsible for the clearing off their driveways and building entrances. The owners may be required to use a de-icing agent such as salt or calcium chloride to maintain a safe walking surface. The de-icing agent for the walkways and building entrances should be kept on site within a building (i.e. garage).

A snow removal contractor will be hired by the subject property owners to maintain the roadway. Snow will be stockpiled on-site within open space areas adjacent to the roadway and not within the wetland resource areas. It will be the responsibility of the hired snow removal contractor to properly dispose of the snow on-site according to the Massachusetts DEP, Snow Disposal Guidelines and all applicable laws and regulations.

ATTACHMENT D

OPERATION & MAINTENANCE PLAN

OPERATION & MAINTENANCE PLAN

SYSTEM OWNER:

North Shore Property Group, LLC (or Successor)

PARTY RESPONSIBLE FOR O&M:

The General Contractor will be responsible for the construction phase operation and maintenance plan. Following construction, lot owners will be responsible for long-term operation and maintenance of the site as outlined in this Operation and Maintenance (O&M) Plan. Should ownership of the property change, the succeeding owner shall assume all responsibilities for implementing this Operation and Maintenance Plan. The lot owners will contract-out maintenance of common utilities and stormwater BMPs.

Note: The system inspectors should note that drainage pipes, manholes, and other drainage structures are considered “confined spaces” subject to strict OSHA standards regarding safe entry. Confined spaces present inherent hazards to workers. Only appropriately trained staff with appropriate safety equipment and monitors may enter confined spaces, and then only with a specific entry permit. Also, this work may pose hazards to workers, such as soft ground, flowing or standing water, snakes and rodents. Again, only appropriately trained staff with the necessary safety equipment should undertake such work.

EMERGENCY CONTACT INFORMATION:

- | | |
|--|----------------|
| • Hamilton Fire & Police (Emergency) | 911 |
| • Hamilton Fire Department (Business) | 1-978-468-5560 |
| • Hamilton Police Department (Business) | 1-978-468-1212 |
| • Hamilton Conservation Commission | 1-978-626-5247 |
| • Hamilton Health Department | 1-978-626-5245 |
| • Hamilton Public Works Department (Emergency) | 1-978-468-4421 |
| • Mass DEP (Emergency Response) | 1-888-304-1133 |
| • Mass DEP (Northeast Regional Office) | 1-978-694-3200 |

CONSTRUCTION PERIOD POLLUTION PREVENTION AND EROSION AND SEDIMENT CONTROL

Installing and maintaining comprehensive erosion controls during construction is critical to the protection of the environment. Following is a list of erosion control measures to be taken to protect the existing and proposed drainage features during construction.

- 1) Install straw wattles or mulch sock downhill of all proposed work area as shown on the Erosion & Sediment Control Plan.
- 2) Install a crushed stone construction entrance.
- 3) Temporary soil stockpiles shall be encircled with straw wattles or mulch sock and stabilized with erosion control matting or temporary seeding, depending upon the proposed duration.
- 4) Stabilize all disturbed areas as soon as practicable. Provide temporary stabilization of soil to be exposed for a long period of time (i.e. one month).
- 5) Sediment accumulation up-gradient of the erosion controls (straw wattles or silt fence) greater than 6" in depth shall be removed and disposed of in accordance with all applicable regulations.
- 6) The stabilized construction entrance shall be inspected weekly. The entrance(s) shall be maintained by adding additional clean, angular stone to remove the soil from the construction vehicle tires. If soil is still observed leaving the site from construction vehicle tires, adjacent roadways shall be kept clean by street sweeping.
- 7) Dust shall be controlled using on-site water as required.
- 8) All erosion control measures shall be maintained, repaired or replaced as required or directed by the owner's engineer or the Hamilton Conservation Commission.

TABLE 1: Construction Phase Inspection and Maintenance Procedures

Control	Inspection Frequency (1)	Maintenance Procedure
Construction Entrance	Weekly	a
Straw Wattle / Mulch Sock	Weekly	a
Stormwater Control Berm	Weekly	a
Dust Control	Daily	b
Permanent Stabilization	Weekly	c

1. Inspection frequencies are a minimum. Site conditions may warrant more frequent review. All control shall be inspected after each storm event which exceeds 0.5 inches in 24-hours.
2. Maintenance Procedures shall be reviewed and revised as necessary to protect the environment.
 - a. Remove accumulated debris and replace as necessary.
 - b. Water or calcium chloride shall be utilized to prevent the generation of dust.
 - c. Disturbed areas shall either be paved or stabilized by permanent seeding.

The attached Construction Phase Maintenance Inspection Form shall be completed weekly and retained with project files.

POST-DEVELOPMENT OPERATION AND MAINTENANCE OF STORMWATER MANAGEMENT SYSTEM

The stormwater management system is four catch basins, two drain manhole, a proprietary device (First Defense Treatment Device), infiltration basin, infiltration trenches, grass swale, and 1,000-gallon drywells.

Inspection of the drainage system components are to be performed by the System Owner during the first year of operation on a quarterly basis. The inspection frequency can be reduced after the first year to annual inspections provided that the quarterly inspections do not indicate the need for more frequent inspections. If more frequent inspections become appropriate at any time, they should be implemented. Inspections should be documented by taking necessary notes, measurements, photographs, and retaining service receipts. The following inspections are required of the system owner.

Paved Surfaces.

As part of normal site clean-up and as discussed in the Long-Term Pollution Prevention Plan, debris is to be removed from the roadway as it accumulates.

Weekly patrolling for litter is recommended. Significant oil leaks should be swept up and disposed of using oil-absorbent materials as they are discovered. Any oil spills or leaks that reach the catch basins must be reported to the Massachusetts DEP oil spill hotline.

Catch basin & Drain Manholes.

Remove the grate or cover and visually inspect for corrosion and structural damage. Inspect pipe inlets and bottoms for signs of infiltration or inflow. The grate or cover and hoods on the catch basins should be inspected on a quarterly basis during the first year and semi-annual thereafter. Cleaning of the catch basins should be done on a yearly basis and by a vacuum truck or clamshell. The contractor is to take care to avoid damaging the structures. While cleaning, if a layer of oil is observed floating on the water surface, place an oil-absorbent pillow on the surface, allow to soak and remove. Repeat this process until the oil layer is removed. Alternatively, have the oil layer pumped out by a licensed disposal contractor and appropriately disposed of. The oil absorbent pillows must be drummed for disposal by a licensed disposal contractor.

First Defense Stormwater Treatment Device.

A copy of the manufacturer's Operation & Maintenance Manual is provided at the end of this section which contains the required operation and maintenance procedures for the treatment devices. The devices should be inspected for corrosion and structural damage on a quarterly basis during the first year and semi-annually thereafter. The device also should be cleaned on a yearly basis and in accordance with the manufacturer's procedural recommendations.

Grass Channel

During the first year, the grass channel should be inspected after major storms to ensure proper function and stabilization. Water levels should be recorded over several days. After the first year, the swale should be inspected twice per year for erosion, sediment accumulation, subsidence, cracking on the embankments. All trash and debris should be removed from the swale and surrounding area. The grass swale should be mowed at least twice annually. Outlet structure should be inspected for sediment accumulation around structure, damage, evidence of clogging or discharge velocities that are greater than design flow. Repair the outlet structure if not plumb, or if drain holes have been damaged

Dry well.

Remove the manhole cover and visually inspect for corrosion, structural damage, silt buildup, and water levels. Initially inspect the dry wells after each major storm event (greater than 1-inch) to ensure proper function and stabilization. Record the water level over several hours to check infiltration performance. After the first year, inspect

the dry wells on a semi-annual basis. Inspect for apparent signs of silt buildup or clogging.

Infiltration Basin.

The infiltration basin should be inspected at least twice per year for erosion, damage to the outlet structure, sediment accumulation around the outlet, subsidence, and cracking or tree growth on the embankments. Inspect the basin outlet structure for evidence of clogging or discharge velocities that are greater than design flow. Repair the outlet structure if not plumb, or if drain holes have been damaged. Remove trash and debris from the pond. The side slopes and the bottoms should be mowed at least twice annually. Sediment should be removed when it exceeds 6-inches in depth. After an extended period of dry weather inspect for standing water. Remove silt from the rip rap areas as it accumulates.

Infiltration Trench.

Initially, inspect the infiltration trench after major storms to ensure proper function and stabilization. Measure and record water levels in the inspection ports over several days to check for the infiltration trench performance. After the first year, inspect the infiltration trench annually for silt buildup or clogging.

Snow Storage Area – The edge of the roadway has been designated as snow storage areas. Reasonable travel lanes shall be maintained. Do not obstruct the visibility of vehicles entering or exiting the site. Debris from melted snow shall be cleared from the site and properly disposed of at the end of the snow season.

In the event snow removal becomes necessary, DEP Snow Disposal Guidance suggest the following restrictions for this site:

- Snow shall not be dumped in wetland resource areas.
- Snow locations shall have at least 50-feet of vegetation within which to filter sediment prior to reaching the drainage system.
- Snow from off-site locations shall not be brought to the site.

RESIDENTIAL SUBDIVISION – 10 MEYER LANE, HAMILTON, MA

CONSTRUCTION PHASE MAINTENANCE INSPECTION FORM

Inspections/reports must be completed at least once every 14 calendar days AND within 24 hours following a rainfall event of 0.50 inches or greater.

Inspection Type: ☐ **Routine (every 14 calendar days)**
☐ **Routine (within 24 hours of 0.50 inches or greater of rainfall)**

Date of Inspection: _____

Weather/Storm Event Information:

Storm Start Time: _____

Storm Duration: _____

Time Elapsed Since Last Storm: _____

Approximate Amount of Rainfall (inches): _____

Weather Conditions during site inspection: _____

Site Notes:

CONSTRUCTION PHASE INSPECTION FORM

BMP Inspection Areas	Sediment Buildup / Sediment Removal Needed?	Need Replacement	Satisfactory or Unsatisfactory	Comments/Corrective Action Required
Construction Entrance/Exit				
Street Entrance Condition				
Silt Fence and/or Straw Wattles Condition				
Construction laydown area				
Material Storage Areas				
Waste Storage Areas				
Concrete Washout				
Temporary Sedimentation Trap (if Applicable)				
Drainage Swale with Checkdams (if Applicable)				
Dust				
Housekeeping / Trash				
Temporary Dewatering Areas				
Other Necessary Controls:				

Description of any discharges
occurring at the time of
inspection

RESIDENTIAL SUBDIVISION – 10 MEYER LANE, HAMILTON, MA
POST-CONSTRUCTION MAINTENANCE INSPECTION FORM

Inspections/reports must be completed quarterly for the first year of operation. Inspection frequency can be reduced after the first year to annual inspections provided that the quarterly inspections do not indicate the need for more frequent inspections.

Inspection Type: ☐ **Quarterly Inspection**
☐ **Annually Inspection**
☐ **Emergency Inspection (i.e. Ponding, Flooding, and/or Severe Erosion)**

Date of Inspection: _____

Weather Conditions during site inspection: _____

Site Notes:

POST-CONSTRUCTION INSPECTION FORM

BMP Inspection Areas	Depth of Sediment Buildup / Sediment Need Removal	Water Level	General Condition	Comments/Corrective Action Required
First Defense Device DMH3				
Catch Basin CB1				
Catch Basin CB2				
Catch Basin CB3				
Catch Basin CB4				
Drain Manhole DMH1				
Drain Manhole DMH 2				
Infiltration Basin				
- Basin Outlet Structure				
- Basin Rip-Rap Level Spreader				
- Emergency Spillway				
Drywells				
Roof Leaders				
Housekeeping / Trash				
Infiltration Trenches				

Site Notes

ATTACHMENT E
ILLICIT DISCHARGE STATEMENT

ILLICIT DISCHARGE COMPLIANCE STATEMENT

I certify that no illicit discharges exist from the 10 Meyer Lane Residential Subdivision and through the implementation of pollution prevention plan measures are set forth to prevent illicit discharges from entering the stormwater management drainage system.

Signature

Print Name

Date

Title

Company

Signature

Print Name

Date

Title

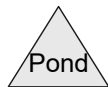
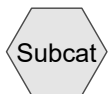
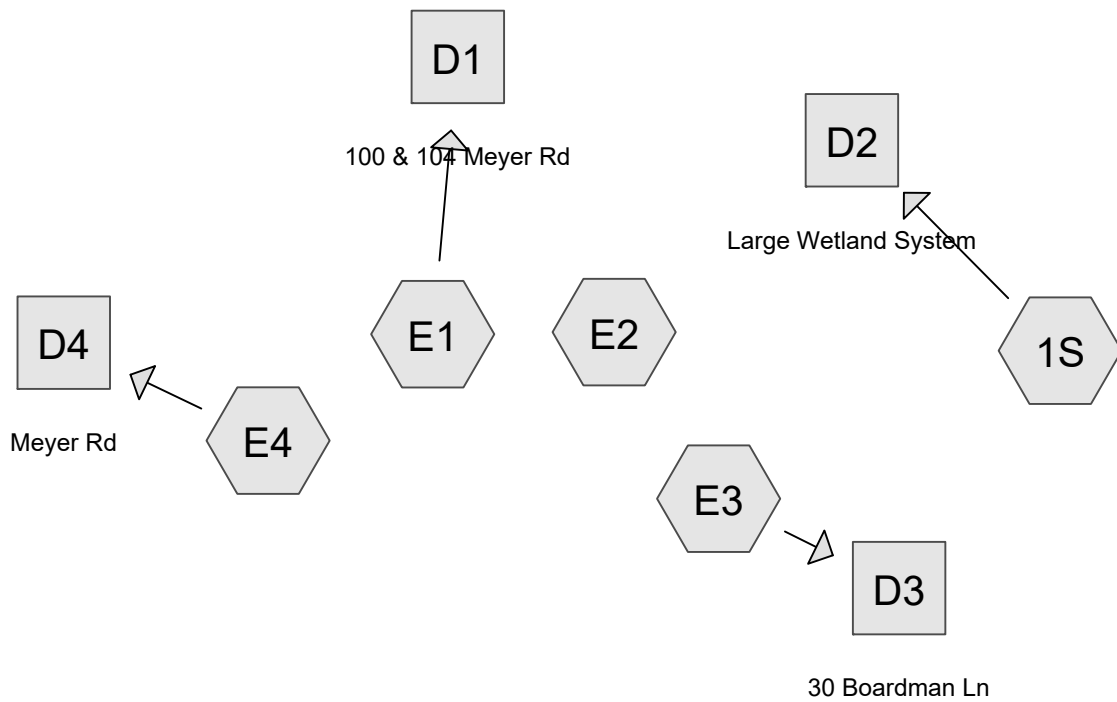
Company

Note: This certification must be signed before stormwater is conveyed to the proposed stormwater drainage system in accordance with Standard 10 of the Massachusetts Stormwater Management Standards.

ATTACHMENT F

STORMWATER ANALYSIS

- **Subcatchment Plans, DR-1 & DR-2
(Reduced Size, 11x17)**
- **HydroCAD Calculations
(2-, 10-, 25-, & 100-Yr Storm Events)**



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Page 2

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-yr	Type III 24-hr		Default	24.00	1	3.10	2
2	10-yr	Type III 24-hr		Default	24.00	1	4.50	2
3	25-yr	Type III 24-hr		Default	24.00	1	5.40	2
4	100-yr	Type III 24-hr		Default	24.00	1	6.50	2

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EXISTING

Type III 24-hr 2-yr Rainfall=3.10"

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Summary for Subcatchment 1S:

Runoff = 2.11 cfs @ 12.32 hrs, Volume= 11,578 cf, Depth> 0.63"
 Routed to Reach D2 : Large Wetland System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
	178,783	60	Woods, Fair, HSG B
	220,029	67	Weighted Average
	178,783		81.25% Pervious Area
	41,246		18.75% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
18.7	685	Total			

Summary for Subcatchment E1:

Runoff = 2.95 cfs @ 12.30 hrs, Volume= 14,141 cf, Depth> 1.08"
 Routed to Reach D1 : 100 & 104 Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	3,615	98	Roofs, HSG B
*	4,344	98	Paved parking, HSG B
	4,913	85	Gravel roads, HSG B
*	3,302	98	Patio, HSG B
	36,685	61	>75% Grass cover, Good, HSG B
*	50,348	98	Wetland B
	54,309	60	Woods, Fair, HSG B
	157,516	76	Weighted Average
	95,907		60.89% Pervious Area
	61,609		39.11% Impervious Area

Buonfiglio-10MeyerLn-Exist

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EXISTING

Type III 24-hr 2-yr Rainfall=3.10"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	100	0.1100	0.15		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
7.8	455	0.0380	0.97		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
0.2	26	0.0050	2.45	0.85	Pipe Channel, 8"Pipe 8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17' n= 0.013 Corrugated PE, smooth interior
1.1	59	0.0300	0.87		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
20.1	640	Total			

Summary for Subcatchment E2:

Runoff = 3.80 cfs @ 12.28 hrs, Volume= 18,600 cf, Depth> 0.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
	178,783	60	Woods, Fair, HSG B
*	38,423	98	Wetland A
	258,452	72	Weighted Average
	178,783		69.17% Pervious Area
	79,669		30.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
18.7	685	Total			

Summary for Subcatchment E3:Runoff = 0.15 cfs @ 12.24 hrs, Volume= 661 cf, Depth> 0.92"
Routed to Reach D3 : 30 Boardman LnRunoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	3,012	98	Wetland B
	5,660	60	Woods, Fair, HSG B
	8,672	73	Weighted Average
	5,660		65.27% Pervious Area
	3,012		34.73% Impervious Area

Buonfiglio-10MeyerLn-Exist

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EXISTING

Type III 24-hr 2-yr Rainfall=3.10"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0500	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
1.0	69	0.0500	1.12		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
16.1	169	Total			

Summary for Subcatchment E4:

Runoff = 0.29 cfs @ 12.09 hrs, Volume= 925 cf, Depth> 1.26"
 Routed to Reach D4 : Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

Area (sf)	CN	Description
3,679	98	Paved parking, HSG B
5,124	65	Woods/grass comb., Fair, HSG B
8,803	79	Weighted Average
5,124		58.21% Pervious Area
3,679		41.79% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.8	100	0.0650	2.17		Sheet Flow, Pavement Smooth surfaces n= 0.011 P2= 3.10"
0.6	177	0.0650	5.18		Shallow Concentrated Flow, Pavement Paved Kv= 20.3 fps
4.6					Direct Entry, Min Tc
6.0	277	Total			

Summary for Reach D1: 100 & 104 Meyer Rd

Inflow Area = 157,516 sf, 39.11% Impervious, Inflow Depth > 1.08" for 2-yr event
 Inflow = 2.95 cfs @ 12.30 hrs, Volume= 14,141 cf
 Outflow = 2.95 cfs @ 12.30 hrs, Volume= 14,141 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D2: Large Wetland System

Inflow Area = 220,029 sf, 18.75% Impervious, Inflow Depth > 0.63" for 2-yr event
 Inflow = 2.11 cfs @ 12.32 hrs, Volume= 11,578 cf
 Outflow = 2.11 cfs @ 12.32 hrs, Volume= 11,578 cf, Atten= 0%, Lag= 0.0 min

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Type III 24-hr 2-yr Rainfall=3.10"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D3: 30 Boardman Ln

Inflow Area = 8,672 sf, 34.73% Impervious, Inflow Depth > 0.92" for 2-yr event
Inflow = 0.15 cfs @ 12.24 hrs, Volume= 661 cf
Outflow = 0.15 cfs @ 12.24 hrs, Volume= 661 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D4: Meyer Rd

Inflow Area = 8,803 sf, 41.79% Impervious, Inflow Depth > 1.26" for 2-yr event
Inflow = 0.29 cfs @ 12.09 hrs, Volume= 925 cf
Outflow = 0.29 cfs @ 12.09 hrs, Volume= 925 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Type III 24-hr 10-yr Rainfall=4.50"

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Summary for Subcatchment 1S:

Runoff = 5.66 cfs @ 12.28 hrs, Volume= 26,705 cf, Depth> 1.46"
 Routed to Reach D2 : Large Wetland System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
	178,783	60	Woods, Fair, HSG B
	220,029	67	Weighted Average
	178,783		81.25% Pervious Area
	41,246		18.75% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
18.7	685	Total			

Summary for Subcatchment E1:

Runoff = 6.02 cfs @ 12.28 hrs, Volume= 27,831 cf, Depth> 2.12"
 Routed to Reach D1 : 100 & 104 Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	3,615	98	Roofs, HSG B
*	4,344	98	Paved parking, HSG B
	4,913	85	Gravel roads, HSG B
*	3,302	98	Patio, HSG B
	36,685	61	>75% Grass cover, Good, HSG B
*	50,348	98	Wetland B
	54,309	60	Woods, Fair, HSG B
	157,516	76	Weighted Average
	95,907		60.89% Pervious Area
	61,609		39.11% Impervious Area

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Type III 24-hr 10-yr Rainfall=4.50"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	100	0.1100	0.15		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
7.8	455	0.0380	0.97		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
0.2	26	0.0050	2.45	0.85	Pipe Channel, 8"Pipe 8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17' n= 0.013 Corrugated PE, smooth interior
1.1	59	0.0300	0.87		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
20.1	640	Total			

Summary for Subcatchment E2:

Runoff = 8.57 cfs @ 12.27 hrs, Volume= 39,029 cf, Depth> 1.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
	178,783	60	Woods, Fair, HSG B
*	38,423	98	Wetland A
	258,452	72	Weighted Average
	178,783		69.17% Pervious Area
	79,669		30.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
18.7	685	Total			

Summary for Subcatchment E3:Runoff = 0.32 cfs @ 12.22 hrs, Volume= 1,365 cf, Depth> 1.89"
Routed to Reach D3 : 30 Boardman LnRunoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	3,012	98	Wetland B
	5,660	60	Woods, Fair, HSG B
	8,672	73	Weighted Average
	5,660		65.27% Pervious Area
	3,012		34.73% Impervious Area

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Type III 24-hr 10-yr Rainfall=4.50"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0500	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
1.0	69	0.0500	1.12		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
16.1	169	Total			

Summary for Subcatchment E4:

Runoff = 0.56 cfs @ 12.09 hrs, Volume= 1,741 cf, Depth> 2.37"
 Routed to Reach D4 : Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

Area (sf)	CN	Description
3,679	98	Paved parking, HSG B
5,124	65	Woods/grass comb., Fair, HSG B
8,803	79	Weighted Average
5,124		58.21% Pervious Area
3,679		41.79% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.8	100	0.0650	2.17		Sheet Flow, Pavement Smooth surfaces n= 0.011 P2= 3.10"
0.6	177	0.0650	5.18		Shallow Concentrated Flow, Pavement Paved Kv= 20.3 fps
4.6					Direct Entry, Min Tc
6.0	277	Total			

Summary for Reach D1: 100 & 104 Meyer Rd

Inflow Area = 157,516 sf, 39.11% Impervious, Inflow Depth > 2.12" for 10-yr event
 Inflow = 6.02 cfs @ 12.28 hrs, Volume= 27,831 cf
 Outflow = 6.02 cfs @ 12.28 hrs, Volume= 27,831 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D2: Large Wetland System

Inflow Area = 220,029 sf, 18.75% Impervious, Inflow Depth > 1.46" for 10-yr event
 Inflow = 5.66 cfs @ 12.28 hrs, Volume= 26,705 cf
 Outflow = 5.66 cfs @ 12.28 hrs, Volume= 26,705 cf, Atten= 0%, Lag= 0.0 min

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Type III 24-hr 10-yr Rainfall=4.50"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D3: 30 Boardman Ln

Inflow Area = 8,672 sf, 34.73% Impervious, Inflow Depth > 1.89" for 10-yr event
Inflow = 0.32 cfs @ 12.22 hrs, Volume= 1,365 cf
Outflow = 0.32 cfs @ 12.22 hrs, Volume= 1,365 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D4: Meyer Rd

Inflow Area = 8,803 sf, 41.79% Impervious, Inflow Depth > 2.37" for 10-yr event
Inflow = 0.56 cfs @ 12.09 hrs, Volume= 1,741 cf
Outflow = 0.56 cfs @ 12.09 hrs, Volume= 1,741 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Type III 24-hr 25-yr Rainfall=5.40"

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Summary for Subcatchment 1S:

Runoff = 8.31 cfs @ 12.28 hrs, Volume= 38,087 cf, Depth> 2.08"
 Routed to Reach D2 : Large Wetland System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
	178,783	60	Woods, Fair, HSG B
	220,029	67	Weighted Average
	178,783		81.25% Pervious Area
	41,246		18.75% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
18.7	685	Total			

Summary for Subcatchment E1:

Runoff = 8.17 cfs @ 12.28 hrs, Volume= 37,496 cf, Depth> 2.86"
 Routed to Reach D1 : 100 & 104 Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	3,615	98	Roofs, HSG B
*	4,344	98	Paved parking, HSG B
	4,913	85	Gravel roads, HSG B
*	3,302	98	Patio, HSG B
	36,685	61	>75% Grass cover, Good, HSG B
*	50,348	98	Wetland B
	54,309	60	Woods, Fair, HSG B
	157,516	76	Weighted Average
	95,907		60.89% Pervious Area
	61,609		39.11% Impervious Area

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Type III 24-hr 25-yr Rainfall=5.40"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	100	0.1100	0.15		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
7.8	455	0.0380	0.97		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
0.2	26	0.0050	2.45	0.85	Pipe Channel, 8"Pipe 8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17' n= 0.013 Corrugated PE, smooth interior
1.1	59	0.0300	0.87		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
20.1	640	Total			

Summary for Subcatchment E2:

Runoff = 11.99 cfs @ 12.26 hrs, Volume= 53,838 cf, Depth> 2.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
	178,783	60	Woods, Fair, HSG B
*	38,423	98	Wetland A
	258,452	72	Weighted Average
	178,783		69.17% Pervious Area
	79,669		30.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
18.7	685	Total			

Summary for Subcatchment E3:Runoff = 0.44 cfs @ 12.22 hrs, Volume= 1,871 cf, Depth> 2.59"
Routed to Reach D3 : 30 Boardman LnRunoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	3,012	98	Wetland B
	5,660	60	Woods, Fair, HSG B
	8,672	73	Weighted Average
	5,660		65.27% Pervious Area
	3,012		34.73% Impervious Area

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Type III 24-hr 25-yr Rainfall=5.40"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0500	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
1.0	69	0.0500	1.12		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
16.1	169	Total			

Summary for Subcatchment E4:

Runoff = 0.75 cfs @ 12.09 hrs, Volume= 2,307 cf, Depth> 3.15"
Routed to Reach D4 : Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-yr Rainfall=5.40"

Area (sf)	CN	Description
3,679	98	Paved parking, HSG B
5,124	65	Woods/grass comb., Fair, HSG B
8,803	79	Weighted Average
5,124		58.21% Pervious Area
3,679		41.79% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.8	100	0.0650	2.17		Sheet Flow, Pavement Smooth surfaces n= 0.011 P2= 3.10"
0.6	177	0.0650	5.18		Shallow Concentrated Flow, Pavement Paved Kv= 20.3 fps
4.6					Direct Entry, Min Tc
6.0	277	Total			

Summary for Reach D1: 100 & 104 Meyer Rd

Inflow Area = 157,516 sf, 39.11% Impervious, Inflow Depth > 2.86" for 25-yr event
Inflow = 8.17 cfs @ 12.28 hrs, Volume= 37,496 cf
Outflow = 8.17 cfs @ 12.28 hrs, Volume= 37,496 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D2: Large Wetland System

Inflow Area = 220,029 sf, 18.75% Impervious, Inflow Depth > 2.08" for 25-yr event
Inflow = 8.31 cfs @ 12.28 hrs, Volume= 38,087 cf
Outflow = 8.31 cfs @ 12.28 hrs, Volume= 38,087 cf, Atten= 0%, Lag= 0.0 min

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Type III 24-hr 25-yr Rainfall=5.40"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D3: 30 Boardman Ln

Inflow Area = 8,672 sf, 34.73% Impervious, Inflow Depth > 2.59" for 25-yr event
Inflow = 0.44 cfs @ 12.22 hrs, Volume= 1,871 cf
Outflow = 0.44 cfs @ 12.22 hrs, Volume= 1,871 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D4: Meyer Rd

Inflow Area = 8,803 sf, 41.79% Impervious, Inflow Depth > 3.15" for 25-yr event
Inflow = 0.75 cfs @ 12.09 hrs, Volume= 2,307 cf
Outflow = 0.75 cfs @ 12.09 hrs, Volume= 2,307 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Type III 24-hr 100-yr Rainfall=6.50"

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Summary for Subcatchment 1S:

Runoff = 11.80 cfs @ 12.27 hrs, Volume= 53,186 cf, Depth> 2.90"
 Routed to Reach D2 : Large Wetland System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
	178,783	60	Woods, Fair, HSG B
	220,029	67	Weighted Average
	178,783		81.25% Pervious Area
	41,246		18.75% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
18.7	685	Total			

Summary for Subcatchment E1:

Runoff = 10.88 cfs @ 12.27 hrs, Volume= 49,882 cf, Depth> 3.80"
 Routed to Reach D1 : 100 & 104 Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	3,615	98	Roofs, HSG B
*	4,344	98	Paved parking, HSG B
	4,913	85	Gravel roads, HSG B
*	3,302	98	Patio, HSG B
	36,685	61	>75% Grass cover, Good, HSG B
*	50,348	98	Wetland B
	54,309	60	Woods, Fair, HSG B
	157,516	76	Weighted Average
	95,907		60.89% Pervious Area
	61,609		39.11% Impervious Area

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Type III 24-hr 100-yr Rainfall=6.50"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	100	0.1100	0.15		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
7.8	455	0.0380	0.97		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
0.2	26	0.0050	2.45	0.85	Pipe Channel, 8"Pipe 8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17' n= 0.013 Corrugated PE, smooth interior
1.1	59	0.0300	0.87		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
20.1	640	Total			

Summary for Subcatchment E2:

Runoff = 16.39 cfs @ 12.26 hrs, Volume= 73,087 cf, Depth> 3.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
	178,783	60	Woods, Fair, HSG B
*	38,423	98	Wetland A
	258,452	72	Weighted Average
	178,783		69.17% Pervious Area
	79,669		30.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
18.7	685	Total			

Summary for Subcatchment E3:Runoff = 0.60 cfs @ 12.22 hrs, Volume= 2,527 cf, Depth> 3.50"
Routed to Reach D3 : 30 Boardman LnRunoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	3,012	98	Wetland B
	5,660	60	Woods, Fair, HSG B
	8,672	73	Weighted Average
	5,660		65.27% Pervious Area
	3,012		34.73% Impervious Area

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Type III 24-hr 100-yr Rainfall=6.50"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0500	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
1.0	69	0.0500	1.12		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
16.1	169	Total			

Summary for Subcatchment E4:

Runoff = 0.97 cfs @ 12.09 hrs, Volume= 3,026 cf, Depth> 4.12"
Routed to Reach D4 : Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=6.50"

Area (sf)	CN	Description
3,679	98	Paved parking, HSG B
5,124	65	Woods/grass comb., Fair, HSG B
8,803	79	Weighted Average
5,124		58.21% Pervious Area
3,679		41.79% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.8	100	0.0650	2.17		Sheet Flow, Pavement Smooth surfaces n= 0.011 P2= 3.10"
0.6	177	0.0650	5.18		Shallow Concentrated Flow, Pavement Paved Kv= 20.3 fps
4.6					Direct Entry, Min Tc
6.0	277	Total			

Summary for Reach D1: 100 & 104 Meyer Rd

Inflow Area = 157,516 sf, 39.11% Impervious, Inflow Depth > 3.80" for 100-yr event
Inflow = 10.88 cfs @ 12.27 hrs, Volume= 49,882 cf
Outflow = 10.88 cfs @ 12.27 hrs, Volume= 49,882 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D2: Large Wetland System

Inflow Area = 220,029 sf, 18.75% Impervious, Inflow Depth > 2.90" for 100-yr event
Inflow = 11.80 cfs @ 12.27 hrs, Volume= 53,186 cf
Outflow = 11.80 cfs @ 12.27 hrs, Volume= 53,186 cf, Atten= 0%, Lag= 0.0 min

Buonfiglio-10MeyerLn-Exist

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EXISTING

Type III 24-hr 100-yr Rainfall=6.50"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D3: 30 Boardman Ln

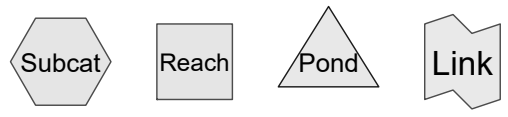
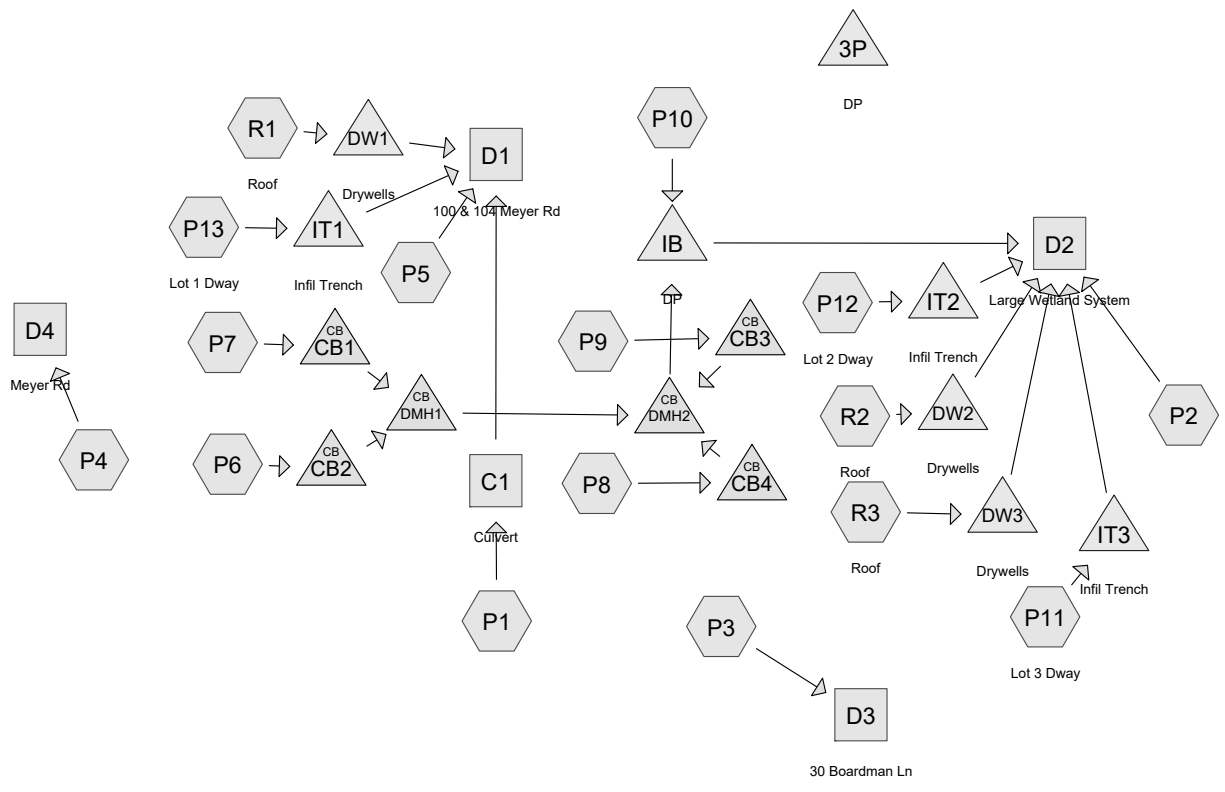
Inflow Area = 8,672 sf, 34.73% Impervious, Inflow Depth > 3.50" for 100-yr event
Inflow = 0.60 cfs @ 12.22 hrs, Volume= 2,527 cf
Outflow = 0.60 cfs @ 12.22 hrs, Volume= 2,527 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D4: Meyer Rd

Inflow Area = 8,803 sf, 41.79% Impervious, Inflow Depth > 4.12" for 100-yr event
Inflow = 0.97 cfs @ 12.09 hrs, Volume= 3,026 cf
Outflow = 0.97 cfs @ 12.09 hrs, Volume= 3,026 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2



Routing Diagram for Buonfiglio-10MeyerLn-Prop
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Buonfiglio-10MeyerLn-Prop

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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-yr	Type III 24-hr		Default	24.00	1	3.10	2
2	10-yr	Type III 24-hr		Default	24.00	1	4.50	2
3	25-yr	Type III 24-hr		Default	24.00	1	5.40	2
4	100-yr	Type III 24-hr		Default	24.00	1	6.50	2

Buonfiglio-10MeyerLn-Prop

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Type III 24-hr 2-yr Rainfall=3.10"

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Summary for Subcatchment P1:

Runoff = 2.54 cfs @ 12.22 hrs, Volume= 10,655 cf, Depth> 1.45"
 Routed to Reach C1 : Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

Area (sf)	CN	Description
2,911	61	>75% Grass cover, Good, HSG B
* 49,898	98	Wetland B
35,191	60	Woods, Fair, HSG B
88,000	82	Weighted Average
38,102		43.30% Pervious Area
49,898		56.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	100	0.1100	0.15		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
4.8	280	0.0380	0.97		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
15.8	380	Total			

Summary for Subcatchment P10:

Runoff = 0.05 cfs @ 12.23 hrs, Volume= 321 cf, Depth> 0.40"
 Routed to Pond IB : DP

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

Area (sf)	CN	Description
3,125	60	Woods, Fair, HSG B
6,457	61	>75% Grass cover, Good, HSG B
9,582	61	Weighted Average
9,582		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	100	0.1000	0.15		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
0.2	34	0.3300	2.87		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
11.7	134	Total			

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Type III 24-hr 2-yr Rainfall=3.10"

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Summary for Subcatchment P11: Lot 3 Dway

Runoff = 0.16 cfs @ 12.17 hrs, Volume= 709 cf, Depth> 0.59"
 Routed to Pond IT3 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	2,406	98	Driveway
	7,165	60	Woods, Fair, HSG B
*	222	30	Infil. Trench
	4,597	61	>75% Grass cover, Good, HSG B
	14,390	66	Weighted Average
	11,984		83.28% Pervious Area
	2,406		16.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.8	64	0.0600	0.11		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.10"
0.2	37	0.2000	3.13		Shallow Concentrated Flow, Lawn Short Grass Pasture Kv= 7.0 fps
0.1	16	0.0200	2.87		Shallow Concentrated Flow, Driveway Paved Kv= 20.3 fps
10.1	117	Total			

Summary for Subcatchment P12: Lot 2 Dway

Runoff = 0.05 cfs @ 12.09 hrs, Volume= 147 cf, Depth> 1.91"
 Routed to Pond IT2 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	783	98	Driveway
*	140	30	Infil. Trench
	923	88	Weighted Average
	140		15.17% Pervious Area
	783		84.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

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Type III 24-hr 2-yr Rainfall=3.10"

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Summary for Subcatchment P13: Lot 1 Dway

Runoff = 0.09 cfs @ 12.10 hrs, Volume= 278 cf, Depth> 1.03"
 Routed to Pond IT1 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	1,365	98	Driveway
*	168	30	Infil. Trench
	1,723	61	>75% Grass cover, Good, HSG B
	3,256	75	Weighted Average
	1,891		58.08% Pervious Area
	1,365		41.92% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P2:

Runoff = 3.65 cfs @ 12.28 hrs, Volume= 17,356 cf, Depth> 0.97"
 Routed to Reach D2 : Large Wetland System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
*	38,423	98	Wetland A
	95,767	60	Woods, Fair, HSG B
	39,869	61	>75% Grass cover, Good, HSG B
	215,305	74	Weighted Average
	135,636		63.00% Pervious Area
	79,669		37.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
18.7	685	Total			

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Type III 24-hr 2-yr Rainfall=3.10"

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Summary for Subcatchment P3:

Runoff = 0.15 cfs @ 12.24 hrs, Volume= 661 cf, Depth> 0.92"
 Routed to Reach D3 : 30 Boardman Ln

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

Area (sf)		CN	Description		
*	3,012	98	Wetland B		
	5,660	60	Woods, Fair, HSG B		
	8,672	73	Weighted Average		
	5,660		65.27% Pervious Area		
	3,012		34.73% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0500	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
1.0	69	0.0500	1.12		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
16.1	169	Total			

Summary for Subcatchment P4:

Runoff = 0.38 cfs @ 12.09 hrs, Volume= 1,195 cf, Depth> 2.07"
 Routed to Reach D4 : Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description		
*	5,364	98	Paved parking, HSG B		
	1,551	61	>75% Grass cover, Good, HSG B		
	6,915	90	Weighted Average		
	1,551		22.43% Pervious Area		
	5,364		77.57% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min Tc

Summary for Subcatchment P5:

Runoff = 0.32 cfs @ 12.18 hrs, Volume= 1,894 cf, Depth> 0.40"
 Routed to Reach D1 : 100 & 104 Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

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Type III 24-hr 2-yr Rainfall=3.10"

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	Area (sf)	CN	Description
*	211	98	Walk, HSG B
	21,956	61	>75% Grass cover, Good, HSG B
*	304	98	Wetland B
	34,018	60	Woods, Fair, HSG B
	56,489	61	Weighted Average
	55,974		99.09% Pervious Area
	515		0.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.8	54	0.0200	1.20		Sheet Flow, Driveway Smooth surfaces n= 0.011 P2= 3.10"
5.3	45	0.0200	0.14		Sheet Flow, Lawn Grass: Short n= 0.150 P2= 3.10"
0.6	78	0.1000	2.21		Shallow Concentrated Flow, Lawn Short Grass Pasture Kv= 7.0 fps
2.4	176	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
9.1	353	Total			

Summary for Subcatchment P6:

Runoff = 0.20 cfs @ 12.08 hrs, Volume= 678 cf, Depth> 2.87"
 Routed to Pond CB2 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	2,839	98	Paved roads w/curbs, HSG B
	2,839		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P7:

Runoff = 0.23 cfs @ 12.08 hrs, Volume= 792 cf, Depth> 2.87"
 Routed to Pond CB1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

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Type III 24-hr 2-yr Rainfall=3.10"

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	Area (sf)	CN	Description
*	3,316	98	Paved roads w/curbs, HSG B
	3,316		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P8:

Runoff = 0.23 cfs @ 12.15 hrs, Volume= 855 cf, Depth> 0.97"
 Routed to Pond CB4 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	3,723	98	Paved roads w/curbs, HSG B
	6,854	61	>75% Grass cover, Good, HSG B
	10,577	74	Weighted Average
	6,854		64.80% Pervious Area
	3,723		35.20% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.9	61	0.0700	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
0.6	14	0.5000	0.41		Sheet Flow, Grass Slope Grass: Short n= 0.150 P2= 3.10"
0.3	75	0.0600	4.97		Shallow Concentrated Flow, Roadway Paved Kv= 20.3 fps
9.8	150	Total			

Summary for Subcatchment P9:

Runoff = 0.23 cfs @ 12.09 hrs, Volume= 696 cf, Depth> 1.75"
 Routed to Pond CB3 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

	Area (sf)	CN	Description
*	3,204	98	Paved roads w/curbs, HSG B
	1,575	61	>75% Grass cover, Good, HSG B
	4,779	86	Weighted Average
	1,575		32.96% Pervious Area
	3,204		67.04% Impervious Area

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Type III 24-hr 2-yr Rainfall=3.10"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.5	18	0.0800	0.21		Sheet Flow, Grass
					Grass: Short n= 0.150 P2= 3.10"
0.4	120	0.0700	5.37		Shallow Concentrated Flow, Roadway
					Paved Kv= 20.3 fps
4.1					Direct Entry, Min. Tc
6.0	138	Total			

Summary for Subcatchment R1: Roof

Runoff = 0.19 cfs @ 12.08 hrs, Volume= 669 cf, Depth> 2.87"
 Routed to Pond DW1 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

Area (sf)	CN	Description
* 2,800	98	Roof
2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment R2: Roof

Runoff = 0.19 cfs @ 12.08 hrs, Volume= 669 cf, Depth> 2.87"
 Routed to Pond DW2 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

Area (sf)	CN	Description
* 2,800	98	Roof
2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment R3: Roof

Runoff = 0.19 cfs @ 12.08 hrs, Volume= 669 cf, Depth> 2.87"
 Routed to Pond DW3 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-yr Rainfall=3.10"

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Type III 24-hr 2-yr Rainfall=3.10"

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	Area (sf)	CN	Description
*	2,800	98	Roof
	2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

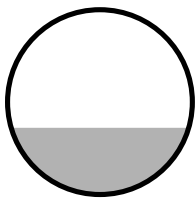
Summary for Reach C1: Culvert

Inflow Area = 88,000 sf, 56.70% Impervious, Inflow Depth > 1.45" for 2-yr event
 Inflow = 2.54 cfs @ 12.22 hrs, Volume= 10,655 cf
 Outflow = 2.54 cfs @ 12.22 hrs, Volume= 10,655 cf, Atten= 0%, Lag= 0.1 min
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Max. Velocity= 9.97 fps, Min. Travel Time= 0.1 min
 Avg. Velocity= 4.03 fps, Avg. Travel Time= 0.1 min

Peak Storage= 9 cf @ 12.22 hrs
 Average Depth at Peak Storage= 0.36' , Surface Width= 0.96'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 9.15 cfs

12.0" Round Pipe
 n= 0.011 Concrete pipe, straight & clean
 Length= 36.0' Slope= 0.0472 '/
 Inlet Invert= 68.30', Outlet Invert= 66.60'

**Summary for Reach D1: 100 & 104 Meyer Rd**

Inflow Area = 150,545 sf, 36.25% Impervious, Inflow Depth > 1.02" for 2-yr event
 Inflow = 2.90 cfs @ 12.22 hrs, Volume= 12,827 cf
 Outflow = 2.90 cfs @ 12.22 hrs, Volume= 12,827 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D2: Large Wetland System

Inflow Area = 267,311 sf, 37.99% Impervious, Inflow Depth > 0.78" for 2-yr event
 Inflow = 3.65 cfs @ 12.28 hrs, Volume= 17,447 cf
 Outflow = 3.65 cfs @ 12.28 hrs, Volume= 17,447 cf, Atten= 0%, Lag= 0.0 min

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Type III 24-hr 2-yr Rainfall=3.10"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D3: 30 Boardman Ln

Inflow Area = 8,672 sf, 34.73% Impervious, Inflow Depth > 0.92" for 2-yr event
 Inflow = 0.15 cfs @ 12.24 hrs, Volume= 661 cf
 Outflow = 0.15 cfs @ 12.24 hrs, Volume= 661 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D4: Meyer Rd

Inflow Area = 6,915 sf, 77.57% Impervious, Inflow Depth > 2.07" for 2-yr event
 Inflow = 0.38 cfs @ 12.09 hrs, Volume= 1,195 cf
 Outflow = 0.38 cfs @ 12.09 hrs, Volume= 1,195 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Pond 3P: DP

Volume	Invert	Avail.Storage	Storage Description
#1A	51.00'	1,235 cf	33.25'W x 39.35'L x 3.75'H Field A 4,906 cf Overall - 1,819 cf Embedded = 3,088 cf x 40.0% Voids
#2A	51.50'	1,819 cf	ADS_StormTech SC-800 +Cap x 35 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 35 Chambers in 7 Rows Cap Storage= 3.4 cf x 2 x 7 rows = 47.9 cf
		3,054 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	51.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	51.00'	
			12.0" Round Culvert L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 51.00' / 47.00' S= 0.1143 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	53.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads 4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#4	Device 2	54.50'	

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Type III 24-hr 2-yr Rainfall=3.10"

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Discarded OutFlow Max=0.00 cfs @ 1.00 hrs HW=0.00' (Free Discharge)↑ **1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=0.00' (Free Discharge)↑ **2=Culvert** (Controls 0.00 cfs)↑ **3=Orifice/Grate** (Controls 0.00 cfs)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond CB1:**

Inflow Area = 3,316 sf, 100.00% Impervious, Inflow Depth > 2.87" for 2-yr event

Inflow = 0.23 cfs @ 12.08 hrs, Volume= 792 cf

Outflow = 0.23 cfs @ 12.08 hrs, Volume= 792 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.23 cfs @ 12.08 hrs, Volume= 792 cf

Routed to Pond DMH1 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 68.06' @ 12.08 hrs

Flood Elev= 70.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	67.85'	12.0" Round Culvert L= 10.7' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.85' / 67.62' S= 0.0215 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.23 cfs @ 12.08 hrs HW=68.06' TW=67.81' (Dynamic Tailwater)↑ **1=Culvert** (Outlet Controls 0.23 cfs @ 2.84 fps)**Summary for Pond CB2:**

Inflow Area = 2,839 sf, 100.00% Impervious, Inflow Depth > 2.87" for 2-yr event

Inflow = 0.20 cfs @ 12.08 hrs, Volume= 678 cf

Outflow = 0.20 cfs @ 12.08 hrs, Volume= 678 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.20 cfs @ 12.08 hrs, Volume= 678 cf

Routed to Pond DMH1 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 68.04' @ 12.08 hrs

Flood Elev= 70.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	67.85'	12.0" Round RCP_Round 12" L= 5.1' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.85' / 67.62' S= 0.0451 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.20 cfs @ 12.08 hrs HW=68.04' TW=67.81' (Dynamic Tailwater)↑ **1=RCP_Round 12"** (Inlet Controls 0.20 cfs @ 1.86 fps)

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Summary for Pond CB3:

Inflow Area = 4,779 sf, 67.04% Impervious, Inflow Depth > 1.75" for 2-yr event
 Inflow = 0.23 cfs @ 12.09 hrs, Volume= 696 cf
 Outflow = 0.23 cfs @ 12.09 hrs, Volume= 696 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.23 cfs @ 12.09 hrs, Volume= 696 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 55.13' @ 12.09 hrs
 Flood Elev= 57.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	54.90'	12.0" Round RCP_Round 12" L= 11.6' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.90' / 54.58' S= 0.0276 ' S= 0.0276 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.22 cfs @ 12.09 hrs HW=55.13' TW=54.94' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Outlet Controls 0.22 cfs @ 2.48 fps)

Summary for Pond CB4:

Inflow Area = 10,577 sf, 35.20% Impervious, Inflow Depth > 0.97" for 2-yr event
 Inflow = 0.23 cfs @ 12.15 hrs, Volume= 855 cf
 Outflow = 0.23 cfs @ 12.15 hrs, Volume= 855 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.23 cfs @ 12.15 hrs, Volume= 855 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 55.12' @ 12.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.90'	12.0" Round RCP_Round 12" L= 6.4' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.90' / 54.58' S= 0.0500 ' S= 0.0500 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.23 cfs @ 12.15 hrs HW=55.11' TW=54.91' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Outlet Controls 0.23 cfs @ 2.80 fps)

Summary for Pond DMH1:

Inflow Area = 6,155 sf, 100.00% Impervious, Inflow Depth > 2.87" for 2-yr event
 Inflow = 0.42 cfs @ 12.08 hrs, Volume= 1,470 cf
 Outflow = 0.42 cfs @ 12.08 hrs, Volume= 1,470 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.42 cfs @ 12.08 hrs, Volume= 1,470 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Peak Elev= 67.81' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.52'	12.0" Round RCP_Round 12" L= 202.7' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.52' / 54.58' S= 0.0638 ' S= 0.0638 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.42 cfs @ 12.08 hrs HW=67.81' TW=54.94' (Dynamic Tailwater)

↑1=RCP_Round 12" (Inlet Controls 0.42 cfs @ 2.28 fps)

Summary for Pond DMH2:

Inflow Area = 21,511 sf, 60.82% Impervious, Inflow Depth > 1.68" for 2-yr event
 Inflow = 0.84 cfs @ 12.10 hrs, Volume= 3,020 cf
 Outflow = 0.84 cfs @ 12.10 hrs, Volume= 3,020 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.84 cfs @ 12.10 hrs, Volume= 3,020 cf
 Routed to Pond IB : DP

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 54.94' @ 12.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.50'	12.0" Round RCP_Round 12" L= 60.8' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.50' / 53.90' S= 0.0099 ' S= 0.0099 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.84 cfs @ 12.10 hrs HW=54.94' TW=52.81' (Dynamic Tailwater)

↑1=RCP_Round 12" (Barrel Controls 0.84 cfs @ 3.75 fps)

Summary for Pond DW1: Drywells

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 2.87" for 2-yr event
 Inflow = 0.19 cfs @ 12.08 hrs, Volume= 669 cf
 Outflow = 0.01 cfs @ 10.92 hrs, Volume= 626 cf, Atten= 94%, Lag= 0.0 min
 Discarded = 0.01 cfs @ 10.92 hrs, Volume= 626 cf
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 2.59' @ 13.90 hrs Surf.Area= 192 sf Storage= 289 cf

Plug-Flow detention time= 219.4 min calculated for 626 cf (94% of inflow)

Center-of-Mass det. time= 184.5 min (941.1 - 756.5)

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Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
209 cf x 2.00 = 418 cf			Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 10.92 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=0.00' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Controls 0.00 cfs)**Summary for Pond DW2: Drywells**

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 2.87" for 2-yr event
 Inflow = 0.19 cfs @ 12.08 hrs, Volume= 669 cf
 Outflow = 0.01 cfs @ 10.92 hrs, Volume= 626 cf, Atten= 94%, Lag= 0.0 min
 Discarded = 0.01 cfs @ 10.92 hrs, Volume= 626 cf
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 2.59' @ 13.90 hrs Surf.Area= 192 sf Storage= 289 cf

Plug-Flow detention time= 219.4 min calculated for 626 cf (94% of inflow)

Center-of-Mass det. time= 184.5 min (941.1 - 756.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
209 cf x 2.00 = 418 cf			Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

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Type III 24-hr 2-yr Rainfall=3.10"

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Discarded OutFlow Max=0.01 cfs @ 10.92 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=0.00' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Controls 0.00 cfs)**Summary for Pond DW3: Drywells**

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 2.87" for 2-yr event
 Inflow = 0.19 cfs @ 12.08 hrs, Volume= 669 cf
 Outflow = 0.01 cfs @ 10.92 hrs, Volume= 626 cf, Atten= 94%, Lag= 0.0 min
 Discarded = 0.01 cfs @ 10.92 hrs, Volume= 626 cf
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 2.59' @ 13.90 hrs Surf.Area= 192 sf Storage= 289 cf

Plug-Flow detention time= 219.4 min calculated for 626 cf (94% of inflow)

Center-of-Mass det. time= 184.5 min (941.1 - 756.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
209 cf x 2.00 = 418 cf Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 10.92 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=0.00' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Controls 0.00 cfs)**Summary for Pond IB: DP**

Inflow Area = 31,093 sf, 42.07% Impervious, Inflow Depth > 1.29" for 2-yr event
 Inflow = 0.86 cfs @ 12.10 hrs, Volume= 3,341 cf
 Outflow = 0.07 cfs @ 13.86 hrs, Volume= 3,118 cf, Atten= 91%, Lag= 105.4 min
 Discarded = 0.07 cfs @ 13.86 hrs, Volume= 3,118 cf
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf
 Routed to Reach D2 : Large Wetland System

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 53.52' @ 13.86 hrs Surf.Area= 1,330 sf Storage= 1,420 cf

Plug-Flow detention time= 205.7 min calculated for 3,117 cf (93% of inflow)
 Center-of-Mass det. time= 169.8 min (984.1 - 814.3)

Volume	Invert	Avail.Storage	Storage Description
#1	52.00'	6,144 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
52.00	458	0	0
53.00	1,113	786	786
54.00	1,530	1,322	2,107
55.00	2,004	1,767	3,874
56.00	2,536	2,270	6,144

Device	Routing	Invert	Outlet Devices
#1	Discarded	52.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	50.00'	12.0" Round RCP_Round 12" L= 42.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 50.00' / 47.00' S= 0.0714 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#3	Device 2	54.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	55.00'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Discarded OutFlow Max=0.07 cfs @ 13.86 hrs HW=53.52' (Free Discharge)

↑ **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=52.00' TW=0.00' (Dynamic Tailwater)

↑ **2=RCP_Round 12"** (Passes 0.00 cfs of 5.79 cfs potential flow)

↑ **3=Orifice/Grate** (Controls 0.00 cfs)

↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond IT1: Infil Trench

Inflow Area = 3,256 sf, 41.92% Impervious, Inflow Depth > 1.03" for 2-yr event
 Inflow = 0.09 cfs @ 12.10 hrs, Volume= 278 cf
 Outflow = 0.09 cfs @ 12.10 hrs, Volume= 278 cf, Atten= 0%, Lag= 0.0 min
 Discarded = 0.00 cfs @ 12.73 hrs, Volume= 0 cf
 Primary = 0.09 cfs @ 12.10 hrs, Volume= 278 cf
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Type III 24-hr 2-yr Rainfall=3.10"

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Peak Elev= 80.50' @ 1.00 hrs Surf.Area= 168 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 0.0 min (859.9 - 859.9)

Volume	Invert	Avail.Storage	Storage Description
#1	80.50'	1,185 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 2,962 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.50	168	0	0
83.00	168	420	420
83.50	10,000	2,542	2,962

Device	Routing	Invert	Outlet Devices
#1	Discarded	80.50'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	59.50'	86.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.00 cfs @ 12.73 hrs HW=80.50' (Free Discharge)↑**1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 12.10 hrs HW=80.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Passes 0.00 cfs of 27,476.76 cfs potential flow)**Summary for Pond IT2: Infil Trench**

Inflow Area = 923 sf, 84.83% Impervious, Inflow Depth > 1.91" for 2-yr event
 Inflow = 0.05 cfs @ 12.09 hrs, Volume= 147 cf
 Outflow = 0.01 cfs @ 11.83 hrs, Volume= 147 cf, Atten= 83%, Lag= 0.0 min
 Discarded = 0.01 cfs @ 11.83 hrs, Volume= 147 cf
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 57.25' @ 12.57 hrs Surf.Area= 140 sf Storage= 42 cf

Plug-Flow detention time= 34.2 min calculated for 146 cf (100% of inflow)

Center-of-Mass det. time= 34.0 min (849.7 - 815.7)

Volume	Invert	Avail.Storage	Storage Description
#1	56.50'	1,154 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 2,885 cf Overall x 40.0% Voids

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Type III 24-hr 2-yr Rainfall=3.10"

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
56.50	140	0	0
59.00	140	350	350
59.50	10,000	2,535	2,885

Device	Routing	Invert	Outlet Devices
#1	Discarded	56.50'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	59.50'	72.0' long x 2.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00			
2.50 3.00 3.50			
Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88			
2.85 3.07 3.20 3.32			

Discarded OutFlow Max=0.01 cfs @ 11.83 hrs HW=56.53' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=56.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond IT3: Infil Trench**

Inflow Area = 14,390 sf, 16.72% Impervious, Inflow Depth > 0.59" for 2-yr event
 Inflow = 0.16 cfs @ 12.17 hrs, Volume= 709 cf
 Outflow = 0.04 cfs @ 12.75 hrs, Volume= 642 cf, Atten= 72%, Lag= 35.0 min
 Discarded = 0.01 cfs @ 12.75 hrs, Volume= 552 cf
 Primary = 0.03 cfs @ 12.75 hrs, Volume= 90 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 63.50' @ 12.75 hrs Surf.Area= 266 sf Storage= 222 cf

Plug-Flow detention time= 185.1 min calculated for 642 cf (91% of inflow)

Center-of-Mass det. time= 140.1 min (1,036.2 - 896.1)

Volume	Invert	Avail.Storage	Storage Description
#1	61.00'	1,244 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 3,111 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
61.00	222	0	0
63.50	222	555	555
64.00	10,000	2,556	3,111

Device	Routing	Invert	Outlet Devices
#1	Discarded	61.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	63.50'	110.0' long x 2.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00			
2.50 3.00 3.50			
Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88			

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2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.01 cfs @ 12.75 hrs HW=63.50' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.03 cfs @ 12.75 hrs HW=63.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 0.03 cfs @ 0.12 fps)

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Type III 24-hr 10-yr Rainfall=4.50"

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Summary for Subcatchment P1:

Runoff = 4.63 cfs @ 12.22 hrs, Volume= 19,271 cf, Depth> 2.63"
 Routed to Reach C1 : Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

Area (sf)	CN	Description
2,911	61	>75% Grass cover, Good, HSG B
* 49,898	98	Wetland B
35,191	60	Woods, Fair, HSG B
88,000	82	Weighted Average
38,102		43.30% Pervious Area
49,898		56.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	100	0.1100	0.15		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
4.8	280	0.0380	0.97		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
15.8	380	Total			

Summary for Subcatchment P10:

Runoff = 0.20 cfs @ 12.18 hrs, Volume= 859 cf, Depth> 1.08"
 Routed to Pond IB : DP

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

Area (sf)	CN	Description
3,125	60	Woods, Fair, HSG B
6,457	61	>75% Grass cover, Good, HSG B
9,582	61	Weighted Average
9,582		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	100	0.1000	0.15		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
0.2	34	0.3300	2.87		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
11.7	134	Total			

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Type III 24-hr 10-yr Rainfall=4.50"

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Summary for Subcatchment P11: Lot 3 Dway

Runoff = 0.44 cfs @ 12.15 hrs, Volume= 1,670 cf, Depth> 1.39"
 Routed to Pond IT3 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	2,406	98	Driveway
	7,165	60	Woods, Fair, HSG B
*	222	30	Infil. Trench
	4,597	61	>75% Grass cover, Good, HSG B
	14,390	66	Weighted Average
	11,984		83.28% Pervious Area
	2,406		16.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.8	64	0.0600	0.11		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.10"
0.2	37	0.2000	3.13		Shallow Concentrated Flow, Lawn Short Grass Pasture Kv= 7.0 fps
0.1	16	0.0200	2.87		Shallow Concentrated Flow, Driveway Paved Kv= 20.3 fps
10.1	117	Total			

Summary for Subcatchment P12: Lot 2 Dway

Runoff = 0.08 cfs @ 12.09 hrs, Volume= 246 cf, Depth> 3.19"
 Routed to Pond IT2 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	783	98	Driveway
*	140	30	Infil. Trench
	923	88	Weighted Average
	140		15.17% Pervious Area
	783		84.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

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Type III 24-hr 10-yr Rainfall=4.50"

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Summary for Subcatchment P13: Lot 1 Dway

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 556 cf, Depth> 2.05"
 Routed to Pond IT1 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	1,365	98	Driveway
*	168	30	Infil. Trench
	1,723	61	>75% Grass cover, Good, HSG B
	3,256	75	Weighted Average
	1,891		58.08% Pervious Area
	1,365		41.92% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P2:

Runoff = 7.80 cfs @ 12.27 hrs, Volume= 35,235 cf, Depth> 1.96"
 Routed to Reach D2 : Large Wetland System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
*	38,423	98	Wetland A
	95,767	60	Woods, Fair, HSG B
	39,869	61	>75% Grass cover, Good, HSG B
	215,305	74	Weighted Average
	135,636		63.00% Pervious Area
	79,669		37.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
18.7	685	Total			

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Type III 24-hr 10-yr Rainfall=4.50"

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Summary for Subcatchment P3:

Runoff = 0.32 cfs @ 12.22 hrs, Volume= 1,365 cf, Depth> 1.89"
 Routed to Reach D3 : 30 Boardman Ln

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

Area (sf)		CN	Description		
*	3,012	98	Wetland B		
	5,660	60	Woods, Fair, HSG B		
	8,672	73	Weighted Average		
	5,660		65.27% Pervious Area		
	3,012		34.73% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0500	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
1.0	69	0.0500	1.12		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
16.1	169	Total			

Summary for Subcatchment P4:

Runoff = 0.61 cfs @ 12.09 hrs, Volume= 1,955 cf, Depth> 3.39"
 Routed to Reach D4 : Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description		
*	5,364	98	Paved parking, HSG B		
	1,551	61	>75% Grass cover, Good, HSG B		
	6,915	90	Weighted Average		
	1,551		22.43% Pervious Area		
	5,364		77.57% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min Tc

Summary for Subcatchment P5:

Runoff = 1.28 cfs @ 12.14 hrs, Volume= 5,067 cf, Depth> 1.08"
 Routed to Reach D1 : 100 & 104 Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

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Type III 24-hr 10-yr Rainfall=4.50"

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	Area (sf)	CN	Description
*	211	98	Walk, HSG B
	21,956	61	>75% Grass cover, Good, HSG B
*	304	98	Wetland B
	34,018	60	Woods, Fair, HSG B
	56,489	61	Weighted Average
	55,974		99.09% Pervious Area
	515		0.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.8	54	0.0200	1.20		Sheet Flow, Driveway Smooth surfaces n= 0.011 P2= 3.10"
5.3	45	0.0200	0.14		Sheet Flow, Lawn Grass: Short n= 0.150 P2= 3.10"
0.6	78	0.1000	2.21		Shallow Concentrated Flow, Lawn Short Grass Pasture Kv= 7.0 fps
2.4	176	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
9.1	353	Total			

Summary for Subcatchment P6:

Runoff = 0.29 cfs @ 12.08 hrs, Volume= 1,008 cf, Depth> 4.26"
Routed to Pond CB2 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	2,839	98	Paved roads w/curbs, HSG B
	2,839		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P7:

Runoff = 0.33 cfs @ 12.08 hrs, Volume= 1,177 cf, Depth> 4.26"
Routed to Pond CB1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.50"

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Type III 24-hr 10-yr Rainfall=4.50"

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	Area (sf)	CN	Description
*	3,316	98	Paved roads w/curbs, HSG B
	3,316		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P8:

Runoff = 0.49 cfs @ 12.14 hrs, Volume= 1,735 cf, Depth> 1.97"
 Routed to Pond CB4 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	3,723	98	Paved roads w/curbs, HSG B
	6,854	61	>75% Grass cover, Good, HSG B
	10,577	74	Weighted Average
	6,854		64.80% Pervious Area
	3,723		35.20% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.9	61	0.0700	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
0.6	14	0.5000	0.41		Sheet Flow, Grass Slope Grass: Short n= 0.150 P2= 3.10"
0.3	75	0.0600	4.97		Shallow Concentrated Flow, Roadway Paved Kv= 20.3 fps
9.8	150	Total			

Summary for Subcatchment P9:

Runoff = 0.38 cfs @ 12.09 hrs, Volume= 1,195 cf, Depth> 3.00"
 Routed to Pond CB3 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 10-yr Rainfall=4.50"

	Area (sf)	CN	Description
*	3,204	98	Paved roads w/curbs, HSG B
	1,575	61	>75% Grass cover, Good, HSG B
	4,779	86	Weighted Average
	1,575		32.96% Pervious Area
	3,204		67.04% Impervious Area

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Type III 24-hr 10-yr Rainfall=4.50"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.5	18	0.0800	0.21		Sheet Flow, Grass
					Grass: Short n= 0.150 P2= 3.10"
0.4	120	0.0700	5.37		Shallow Concentrated Flow, Roadway
					Paved Kv= 20.3 fps
4.1					Direct Entry, Min. Tc
6.0	138	Total			

Summary for Subcatchment R1: Roof

Runoff = 0.28 cfs @ 12.08 hrs, Volume= 994 cf, Depth> 4.26"
Routed to Pond DW1 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.50"

Area (sf)	CN	Description
* 2,800	98	Roof
2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment R2: Roof

Runoff = 0.28 cfs @ 12.08 hrs, Volume= 994 cf, Depth> 4.26"
Routed to Pond DW2 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.50"

Area (sf)	CN	Description
* 2,800	98	Roof
2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment R3: Roof

Runoff = 0.28 cfs @ 12.08 hrs, Volume= 994 cf, Depth> 4.26"
Routed to Pond DW3 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-yr Rainfall=4.50"

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	Area (sf)	CN	Description
*	2,800	98	Roof
	2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

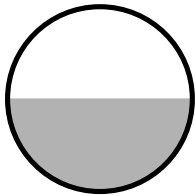
Summary for Reach C1: Culvert

Inflow Area = 88,000 sf, 56.70% Impervious, Inflow Depth > 2.63" for 10-yr event
 Inflow = 4.63 cfs @ 12.22 hrs, Volume= 19,271 cf
 Outflow = 4.63 cfs @ 12.21 hrs, Volume= 19,270 cf, Atten= 0%, Lag= 0.0 min
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Max. Velocity= 11.68 fps, Min. Travel Time= 0.1 min
 Avg. Velocity= 4.60 fps, Avg. Travel Time= 0.1 min

Peak Storage= 14 cf @ 12.21 hrs
 Average Depth at Peak Storage= 0.50' , Surface Width= 1.00'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 9.15 cfs

12.0" Round Pipe
 n= 0.011 Concrete pipe, straight & clean
 Length= 36.0' Slope= 0.0472 '/
 Inlet Invert= 68.30', Outlet Invert= 66.60'

**Summary for Reach D1: 100 & 104 Meyer Rd**

Inflow Area = 150,545 sf, 36.25% Impervious, Inflow Depth > 1.99" for 10-yr event
 Inflow = 5.86 cfs @ 12.20 hrs, Volume= 25,010 cf
 Outflow = 5.86 cfs @ 12.20 hrs, Volume= 25,010 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D2: Large Wetland System

Inflow Area = 267,311 sf, 37.99% Impervious, Inflow Depth > 1.67" for 10-yr event
 Inflow = 8.11 cfs @ 12.26 hrs, Volume= 37,203 cf
 Outflow = 8.11 cfs @ 12.26 hrs, Volume= 37,203 cf, Atten= 0%, Lag= 0.0 min

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D3: 30 Boardman Ln

Inflow Area = 8,672 sf, 34.73% Impervious, Inflow Depth > 1.89" for 10-yr event
 Inflow = 0.32 cfs @ 12.22 hrs, Volume= 1,365 cf
 Outflow = 0.32 cfs @ 12.22 hrs, Volume= 1,365 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D4: Meyer Rd

Inflow Area = 6,915 sf, 77.57% Impervious, Inflow Depth > 3.39" for 10-yr event
 Inflow = 0.61 cfs @ 12.09 hrs, Volume= 1,955 cf
 Outflow = 0.61 cfs @ 12.09 hrs, Volume= 1,955 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Pond 3P: DP

Volume	Invert	Avail.Storage	Storage Description
#1A	51.00'	1,235 cf	33.25'W x 39.35'L x 3.75'H Field A 4,906 cf Overall - 1,819 cf Embedded = 3,088 cf x 40.0% Voids
#2A	51.50'	1,819 cf	ADS_StormTech SC-800 +Cap x 35 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 35 Chambers in 7 Rows Cap Storage= 3.4 cf x 2 x 7 rows = 47.9 cf
		3,054 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	51.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	51.00'	12.0" Round Culvert L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 51.00' / 47.00' S= 0.1143 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	53.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	54.50'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

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Type III 24-hr 10-yr Rainfall=4.50"

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Discarded OutFlow Max=0.00 cfs @ 1.00 hrs HW=0.00' (Free Discharge)↑ **1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=0.00' (Free Discharge)↑ **2=Culvert** (Controls 0.00 cfs)↑ **3=Orifice/Grate** (Controls 0.00 cfs)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond CB1:**

Inflow Area = 3,316 sf, 100.00% Impervious, Inflow Depth > 4.26" for 10-yr event
Inflow = 0.33 cfs @ 12.08 hrs, Volume= 1,177 cf
Outflow = 0.33 cfs @ 12.08 hrs, Volume= 1,177 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.33 cfs @ 12.08 hrs, Volume= 1,177 cf
Routed to Pond DMH1 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 68.12' @ 12.08 hrs

Flood Elev= 70.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	67.85'	12.0" Round Culvert L= 10.7' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.85' / 67.62' S= 0.0215 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.33 cfs @ 12.08 hrs HW=68.12' TW=67.87' (Dynamic Tailwater)↑ **1=Culvert** (Outlet Controls 0.33 cfs @ 2.95 fps)**Summary for Pond CB2:**

Inflow Area = 2,839 sf, 100.00% Impervious, Inflow Depth > 4.26" for 10-yr event
Inflow = 0.29 cfs @ 12.08 hrs, Volume= 1,008 cf
Outflow = 0.29 cfs @ 12.08 hrs, Volume= 1,008 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.29 cfs @ 12.08 hrs, Volume= 1,008 cf
Routed to Pond DMH1 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 68.09' @ 12.08 hrs

Flood Elev= 70.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	67.85'	12.0" Round RCP_Round 12" L= 5.1' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.85' / 67.62' S= 0.0451 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.29 cfs @ 12.08 hrs HW=68.09' TW=67.87' (Dynamic Tailwater)↑ **1=RCP_Round 12"** (Outlet Controls 0.29 cfs @ 3.01 fps)

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Summary for Pond CB3:

Inflow Area = 4,779 sf, 67.04% Impervious, Inflow Depth > 3.00" for 10-yr event
 Inflow = 0.38 cfs @ 12.09 hrs, Volume= 1,195 cf
 Outflow = 0.38 cfs @ 12.09 hrs, Volume= 1,195 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.38 cfs @ 12.09 hrs, Volume= 1,195 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 55.24' @ 12.09 hrs
 Flood Elev= 57.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	54.90'	12.0" Round RCP_Round 12" L= 11.6' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.90' / 54.58' S= 0.0276 ' S= 0.0276 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.38 cfs @ 12.09 hrs HW=55.24' TW=55.09' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Outlet Controls 0.38 cfs @ 2.38 fps)

Summary for Pond CB4:

Inflow Area = 10,577 sf, 35.20% Impervious, Inflow Depth > 1.97" for 10-yr event
 Inflow = 0.49 cfs @ 12.14 hrs, Volume= 1,735 cf
 Outflow = 0.49 cfs @ 12.14 hrs, Volume= 1,735 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.49 cfs @ 12.14 hrs, Volume= 1,735 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 55.26' @ 12.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.90'	12.0" Round RCP_Round 12" L= 6.4' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.90' / 54.58' S= 0.0500 ' S= 0.0500 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.49 cfs @ 12.14 hrs HW=55.26' TW=55.06' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Outlet Controls 0.49 cfs @ 2.90 fps)

Summary for Pond DMH1:

Inflow Area = 6,155 sf, 100.00% Impervious, Inflow Depth > 4.26" for 10-yr event
 Inflow = 0.62 cfs @ 12.08 hrs, Volume= 2,185 cf
 Outflow = 0.62 cfs @ 12.08 hrs, Volume= 2,185 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.62 cfs @ 12.08 hrs, Volume= 2,185 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Type III 24-hr 10-yr Rainfall=4.50"

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Peak Elev= 67.87' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.52'	12.0" Round RCP_Round 12" L= 202.7' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.52' / 54.58' S= 0.0638 ' S= 0.0638 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.62 cfs @ 12.08 hrs HW=67.87' TW=55.09' (Dynamic Tailwater)

↑1=RCP_Round 12" (Inlet Controls 0.62 cfs @ 2.52 fps)

Summary for Pond DMH2:

Inflow Area = 21,511 sf, 60.82% Impervious, Inflow Depth > 2.85" for 10-yr event
Inflow = 1.43 cfs @ 12.10 hrs, Volume= 5,115 cf
Outflow = 1.43 cfs @ 12.10 hrs, Volume= 5,115 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.43 cfs @ 12.10 hrs, Volume= 5,115 cf
Routed to Pond IB : DP

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 55.10' @ 12.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.50'	12.0" Round RCP_Round 12" L= 60.8' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.50' / 53.90' S= 0.0099 ' S= 0.0099 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.43 cfs @ 12.10 hrs HW=55.10' TW=53.39' (Dynamic Tailwater)

↑1=RCP_Round 12" (Barrel Controls 1.43 cfs @ 4.22 fps)

Summary for Pond DW1: Drywells

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 4.26" for 10-yr event
Inflow = 0.28 cfs @ 12.08 hrs, Volume= 994 cf
Outflow = 0.07 cfs @ 12.46 hrs, Volume= 798 cf, Atten= 76%, Lag= 22.6 min
Discarded = 0.01 cfs @ 9.82 hrs, Volume= 681 cf
Primary = 0.06 cfs @ 12.46 hrs, Volume= 117 cf
Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 3.47' @ 12.46 hrs Surf.Area= 192 sf Storage= 404 cf

Plug-Flow detention time= 208.3 min calculated for 798 cf (80% of inflow)

Center-of-Mass det. time= 132.4 min (881.7 - 749.3)

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Type III 24-hr 10-yr Rainfall=4.50"

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Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
			209 cf x 2.00 = 418 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 9.82 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.06 cfs @ 12.46 hrs HW=3.47' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.06 cfs @ 1.28 fps)**Summary for Pond DW2: Drywells**

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 4.26" for 10-yr event
 Inflow = 0.28 cfs @ 12.08 hrs, Volume= 994 cf
 Outflow = 0.07 cfs @ 12.46 hrs, Volume= 798 cf, Atten= 76%, Lag= 22.6 min
 Discarded = 0.01 cfs @ 9.82 hrs, Volume= 681 cf
 Primary = 0.06 cfs @ 12.46 hrs, Volume= 117 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 3.47' @ 12.46 hrs Surf.Area= 192 sf Storage= 404 cf

Plug-Flow detention time= 208.3 min calculated for 798 cf (80% of inflow)

Center-of-Mass det. time= 132.4 min (881.7 - 749.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
			209 cf x 2.00 = 418 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

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Type III 24-hr 10-yr Rainfall=4.50"

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Discarded OutFlow Max=0.01 cfs @ 9.82 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.06 cfs @ 12.46 hrs HW=3.47' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.06 cfs @ 1.28 fps)**Summary for Pond DW3: Drywells**

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 4.26" for 10-yr event
 Inflow = 0.28 cfs @ 12.08 hrs, Volume= 994 cf
 Outflow = 0.07 cfs @ 12.46 hrs, Volume= 798 cf, Atten= 76%, Lag= 22.6 min
 Discarded = 0.01 cfs @ 9.82 hrs, Volume= 681 cf
 Primary = 0.06 cfs @ 12.46 hrs, Volume= 117 cf

Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 3.47' @ 12.46 hrs Surf.Area= 192 sf Storage= 404 cf

Plug-Flow detention time= 208.3 min calculated for 798 cf (80% of inflow)

Center-of-Mass det. time= 132.4 min (881.7 - 749.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
209 cf x 2.00 = 418 cf Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 9.82 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.06 cfs @ 12.46 hrs HW=3.47' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.06 cfs @ 1.28 fps)**Summary for Pond IB: DP**

Inflow Area = 31,093 sf, 42.07% Impervious, Inflow Depth > 2.31" for 10-yr event
 Inflow = 1.58 cfs @ 12.10 hrs, Volume= 5,974 cf
 Outflow = 0.24 cfs @ 12.77 hrs, Volume= 4,993 cf, Atten= 85%, Lag= 40.0 min
 Discarded = 0.09 cfs @ 12.77 hrs, Volume= 4,112 cf
 Primary = 0.15 cfs @ 12.77 hrs, Volume= 881 cf

Routed to Reach D2 : Large Wetland System

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Type III 24-hr 10-yr Rainfall=4.50"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 54.29' @ 12.77 hrs Surf.Area= 1,666 sf Storage= 2,566 cf

Plug-Flow detention time= 217.7 min calculated for 4,991 cf (84% of inflow)

Center-of-Mass det. time= 147.1 min (955.2 - 808.1)

Volume	Invert	Avail.Storage	Storage Description
#1	52.00'	6,144 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
52.00	458	0	0
53.00	1,113	786	786
54.00	1,530	1,322	2,107
55.00	2,004	1,767	3,874
56.00	2,536	2,270	6,144

Device	Routing	Invert	Outlet Devices
#1	Discarded	52.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	50.00'	12.0" Round RCP_Round 12" L= 42.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 50.00' / 47.00' S= 0.0714 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#3	Device 2	54.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	55.00'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Discarded OutFlow Max=0.09 cfs @ 12.77 hrs HW=54.29' (Free Discharge)↑ **1=Exfiltration** (Exfiltration Controls 0.09 cfs)**Primary OutFlow** Max=0.15 cfs @ 12.77 hrs HW=54.29' TW=0.00' (Dynamic Tailwater)↑ **2=RCP_Round 12"** (Passes 0.15 cfs of 9.20 cfs potential flow)↑ **3=Orifice/Grate** (Orifice Controls 0.15 cfs @ 1.83 fps)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond IT1: Infil Trench**

Inflow Area = 3,256 sf, 41.92% Impervious, Inflow Depth > 2.05" for 10-yr event
 Inflow = 0.18 cfs @ 12.09 hrs, Volume= 556 cf
 Outflow = 0.18 cfs @ 12.09 hrs, Volume= 556 cf, Atten= 0%, Lag= 0.0 min
 Discarded = 0.00 cfs @ 13.84 hrs, Volume= 0 cf
 Primary = 0.18 cfs @ 12.09 hrs, Volume= 556 cf
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Type III 24-hr 10-yr Rainfall=4.50"

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Peak Elev= 80.50' @ 1.00 hrs Surf.Area= 168 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= (not calculated: outflow precedes inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	80.50'	1,185 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 2,962 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.50	168	0	0
83.00	168	420	420
83.50	10,000	2,542	2,962

Device	Routing	Invert	Outlet Devices
#1	Discarded	80.50'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	59.50'	86.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.00 cfs @ 13.84 hrs HW=80.50' (Free Discharge)↑**1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=80.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Passes 0.00 cfs of 27,476.76 cfs potential flow)**Summary for Pond IT2: Infil Trench**

Inflow Area = 923 sf, 84.83% Impervious, Inflow Depth > 3.19" for 10-yr event
 Inflow = 0.08 cfs @ 12.09 hrs, Volume= 246 cf
 Outflow = 0.01 cfs @ 11.69 hrs, Volume= 246 cf, Atten= 90%, Lag= 0.0 min
 Discarded = 0.01 cfs @ 11.69 hrs, Volume= 246 cf
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 58.08' @ 12.91 hrs Surf.Area= 140 sf Storage= 88 cf

Plug-Flow detention time= 86.1 min calculated for 246 cf (100% of inflow)

Center-of-Mass det. time= 85.9 min (887.0 - 801.1)

Volume	Invert	Avail.Storage	Storage Description
#1	56.50'	1,154 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 2,885 cf Overall x 40.0% Voids

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Type III 24-hr 10-yr Rainfall=4.50"

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
56.50	140	0	0
59.00	140	350	350
59.50	10,000	2,535	2,885

Device	Routing	Invert	Outlet Devices
#1	Discarded	56.50'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	59.50'	72.0' long x 2.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00			
2.50 3.00 3.50			
Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88			
2.85 3.07 3.20 3.32			

Discarded OutFlow Max=0.01 cfs @ 11.69 hrs HW=56.53' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=56.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond IT3: Infil Trench**

Inflow Area = 14,390 sf, 16.72% Impervious, Inflow Depth > 1.39" for 10-yr event
 Inflow = 0.44 cfs @ 12.15 hrs, Volume= 1,670 cf
 Outflow = 0.45 cfs @ 12.15 hrs, Volume= 1,477 cf, Atten= 0%, Lag= 0.0 min
 Discarded = 0.03 cfs @ 12.15 hrs, Volume= 624 cf
 Primary = 0.42 cfs @ 12.15 hrs, Volume= 853 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 63.51' @ 12.15 hrs Surf.Area= 478 sf Storage= 224 cf

Plug-Flow detention time= 85.8 min calculated for 1,476 cf (88% of inflow)

Center-of-Mass det. time= 31.8 min (898.8 - 867.0)

Volume	Invert	Avail.Storage	Storage Description
#1	61.00'	1,244 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 3,111 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
61.00	222	0	0
63.50	222	555	555
64.00	10,000	2,556	3,111

Device	Routing	Invert	Outlet Devices
#1	Discarded	61.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	63.50'	110.0' long x 2.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00			
2.50 3.00 3.50			
Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88			

Buonfiglio-10MeyerLn-Prop*Type III 24-hr 10-yr Rainfall=4.50"*

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2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.03 cfs @ 12.15 hrs HW=63.51' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.03 cfs)**Primary OutFlow** Max=0.42 cfs @ 12.15 hrs HW=63.51' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 0.42 cfs @ 0.29 fps)

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Type III 24-hr 25-yr Rainfall=5.40"

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Summary for Subcatchment P1:

Runoff = 6.02 cfs @ 12.21 hrs, Volume= 25,146 cf, Depth> 3.43"
 Routed to Reach C1 : Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

Area (sf)	CN	Description
2,911	61	>75% Grass cover, Good, HSG B
* 49,898	98	Wetland B
35,191	60	Woods, Fair, HSG B
88,000	82	Weighted Average
38,102		43.30% Pervious Area
49,898		56.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	100	0.1100	0.15		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
4.8	280	0.0380	0.97		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
15.8	380	Total			

Summary for Subcatchment P10:

Runoff = 0.32 cfs @ 12.18 hrs, Volume= 1,286 cf, Depth> 1.61"
 Routed to Pond IB : DP

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

Area (sf)	CN	Description
3,125	60	Woods, Fair, HSG B
6,457	61	>75% Grass cover, Good, HSG B
9,582	61	Weighted Average
9,582		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	100	0.1000	0.15		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
0.2	34	0.3300	2.87		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
11.7	134	Total			

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Type III 24-hr 25-yr Rainfall=5.40"

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Summary for Subcatchment P11: Lot 3 Dway

Runoff = 0.65 cfs @ 12.15 hrs, Volume= 2,399 cf, Depth> 2.00"
 Routed to Pond IT3 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	2,406	98	Driveway
	7,165	60	Woods, Fair, HSG B
*	222	30	Infil. Trench
	4,597	61	>75% Grass cover, Good, HSG B
	14,390	66	Weighted Average
	11,984		83.28% Pervious Area
	2,406		16.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.8	64	0.0600	0.11		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.10"
0.2	37	0.2000	3.13		Shallow Concentrated Flow, Lawn Short Grass Pasture Kv= 7.0 fps
0.1	16	0.0200	2.87		Shallow Concentrated Flow, Driveway Paved Kv= 20.3 fps
10.1	117	Total			

Summary for Subcatchment P12: Lot 2 Dway

Runoff = 0.10 cfs @ 12.09 hrs, Volume= 311 cf, Depth> 4.05"
 Routed to Pond IT2 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	783	98	Driveway
*	140	30	Infil. Trench
	923	88	Weighted Average
	140		15.17% Pervious Area
	783		84.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

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Type III 24-hr 25-yr Rainfall=5.40"

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Summary for Subcatchment P13: Lot 1 Dway

Runoff = 0.24 cfs @ 12.09 hrs, Volume= 753 cf, Depth> 2.77"
 Routed to Pond IT1 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	1,365	98	Driveway
*	168	30	Infil. Trench
	1,723	61	>75% Grass cover, Good, HSG B
	3,256	75	Weighted Average
	1,891		58.08% Pervious Area
	1,365		41.92% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P2:

Runoff = 10.74 cfs @ 12.26 hrs, Volume= 48,020 cf, Depth> 2.68"
 Routed to Reach D2 : Large Wetland System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
*	38,423	98	Wetland A
	95,767	60	Woods, Fair, HSG B
	39,869	61	>75% Grass cover, Good, HSG B
	215,305	74	Weighted Average
	135,636		63.00% Pervious Area
	79,669		37.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
18.7	685	Total			

Buonfiglio-10MeyerLn-Prop

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Type III 24-hr 25-yr Rainfall=5.40"

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Summary for Subcatchment P3:

Runoff = 0.44 cfs @ 12.22 hrs, Volume= 1,871 cf, Depth> 2.59"
 Routed to Reach D3 : 30 Boardman Ln

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

Area (sf)		CN	Description		
*	3,012	98	Wetland B		
	5,660	60	Woods, Fair, HSG B		
	8,672	73	Weighted Average		
	5,660		65.27% Pervious Area		
	3,012		34.73% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0500	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
1.0	69	0.0500	1.12		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
16.1	169	Total			

Summary for Subcatchment P4:

Runoff = 0.76 cfs @ 12.08 hrs, Volume= 2,454 cf, Depth> 4.26"
 Routed to Reach D4 : Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description		
*	5,364	98	Paved parking, HSG B		
	1,551	61	>75% Grass cover, Good, HSG B		
	6,915	90	Weighted Average		
	1,551		22.43% Pervious Area		
	5,364		77.57% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min Tc

Summary for Subcatchment P5:

Runoff = 2.05 cfs @ 12.14 hrs, Volume= 7,586 cf, Depth> 1.61"
 Routed to Reach D1 : 100 & 104 Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

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	Area (sf)	CN	Description
*	211	98	Walk, HSG B
	21,956	61	>75% Grass cover, Good, HSG B
*	304	98	Wetland B
	34,018	60	Woods, Fair, HSG B
	56,489	61	Weighted Average
	55,974		99.09% Pervious Area
	515		0.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.8	54	0.0200	1.20		Sheet Flow, Driveway Smooth surfaces n= 0.011 P2= 3.10"
5.3	45	0.0200	0.14		Sheet Flow, Lawn Grass: Short n= 0.150 P2= 3.10"
0.6	78	0.1000	2.21		Shallow Concentrated Flow, Lawn Short Grass Pasture Kv= 7.0 fps
2.4	176	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
9.1	353	Total			

Summary for Subcatchment P6:

Runoff = 0.34 cfs @ 12.08 hrs, Volume= 1,220 cf, Depth> 5.16"
 Routed to Pond CB2 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	2,839	98	Paved roads w/curbs, HSG B
	2,839		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P7:

Runoff = 0.40 cfs @ 12.08 hrs, Volume= 1,425 cf, Depth> 5.16"
 Routed to Pond CB1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

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Type III 24-hr 25-yr Rainfall=5.40"

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	Area (sf)	CN	Description
*	3,316	98	Paved roads w/curbs, HSG B
	3,316		100.00% Impervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P8:

Runoff = 0.67 cfs @ 12.14 hrs, Volume= 2,364 cf, Depth> 2.68"
 Routed to Pond CB4 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	3,723	98	Paved roads w/curbs, HSG B
	6,854	61	>75% Grass cover, Good, HSG B
	10,577	74	Weighted Average
	6,854		64.80% Pervious Area
	3,723		35.20% Impervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
8.9	61	0.0700	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
0.6	14	0.5000	0.41		Sheet Flow, Grass Slope Grass: Short n= 0.150 P2= 3.10"
0.3	75	0.0600	4.97		Shallow Concentrated Flow, Roadway Paved Kv= 20.3 fps
9.8	150	Total			

Summary for Subcatchment P9:

Runoff = 0.49 cfs @ 12.09 hrs, Volume= 1,529 cf, Depth> 3.84"
 Routed to Pond CB3 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

	Area (sf)	CN	Description
*	3,204	98	Paved roads w/curbs, HSG B
	1,575	61	>75% Grass cover, Good, HSG B
	4,779	86	Weighted Average
	1,575		32.96% Pervious Area
	3,204		67.04% Impervious Area

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Type III 24-hr 25-yr Rainfall=5.40"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.5	18	0.0800	0.21		Sheet Flow, Grass
					Grass: Short n= 0.150 P2= 3.10"
0.4	120	0.0700	5.37		Shallow Concentrated Flow, Roadway
					Paved Kv= 20.3 fps
4.1					Direct Entry, Min. Tc
6.0	138	Total			

Summary for Subcatchment R1: Roof

Runoff = 0.34 cfs @ 12.08 hrs, Volume= 1,204 cf, Depth> 5.16"
 Routed to Pond DW1 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

Area (sf)	CN	Description
* 2,800	98	Roof
2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment R2: Roof

Runoff = 0.34 cfs @ 12.08 hrs, Volume= 1,204 cf, Depth> 5.16"
 Routed to Pond DW2 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

Area (sf)	CN	Description
* 2,800	98	Roof
2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment R3: Roof

Runoff = 0.34 cfs @ 12.08 hrs, Volume= 1,204 cf, Depth> 5.16"
 Routed to Pond DW3 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-yr Rainfall=5.40"

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Type III 24-hr 25-yr Rainfall=5.40"

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	Area (sf)	CN	Description
*	2,800	98	Roof
	2,800		100.00% Impervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry, Min. Tc

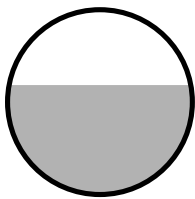
Summary for Reach C1: Culvert

Inflow Area = 88,000 sf, 56.70% Impervious, Inflow Depth > 3.43" for 25-yr event
 Inflow = 6.02 cfs @ 12.21 hrs, Volume= 25,146 cf
 Outflow = 6.02 cfs @ 12.21 hrs, Volume= 25,146 cf, Atten= 0%, Lag= 0.0 min
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Max. Velocity= 12.44 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 4.88 fps, Avg. Travel Time= 0.1 min

Peak Storage= 17 cf @ 12.21 hrs
 Average Depth at Peak Storage= 0.59' , Surface Width= 0.98'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 9.15 cfs

12.0" Round Pipe
 n= 0.011 Concrete pipe, straight & clean
 Length= 36.0' Slope= 0.0472 '/
 Inlet Invert= 68.30', Outlet Invert= 66.60'

**Summary for Reach D1: 100 & 104 Meyer Rd**

Inflow Area = 150,545 sf, 36.25% Impervious, Inflow Depth > 2.69" for 25-yr event
 Inflow = 8.12 cfs @ 12.20 hrs, Volume= 33,755 cf
 Outflow = 8.12 cfs @ 12.20 hrs, Volume= 33,755 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D2: Large Wetland System

Inflow Area = 267,311 sf, 37.99% Impervious, Inflow Depth > 2.35" for 25-yr event
 Inflow = 11.67 cfs @ 12.26 hrs, Volume= 52,285 cf
 Outflow = 11.67 cfs @ 12.26 hrs, Volume= 52,285 cf, Atten= 0%, Lag= 0.0 min

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D3: 30 Boardman Ln

Inflow Area = 8,672 sf, 34.73% Impervious, Inflow Depth > 2.59" for 25-yr event
 Inflow = 0.44 cfs @ 12.22 hrs, Volume= 1,871 cf
 Outflow = 0.44 cfs @ 12.22 hrs, Volume= 1,871 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D4: Meyer Rd

Inflow Area = 6,915 sf, 77.57% Impervious, Inflow Depth > 4.26" for 25-yr event
 Inflow = 0.76 cfs @ 12.08 hrs, Volume= 2,454 cf
 Outflow = 0.76 cfs @ 12.08 hrs, Volume= 2,454 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Pond 3P: DP

Volume	Invert	Avail.Storage	Storage Description
#1A	51.00'	1,235 cf	33.25'W x 39.35'L x 3.75'H Field A 4,906 cf Overall - 1,819 cf Embedded = 3,088 cf x 40.0% Voids
#2A	51.50'	1,819 cf	ADS_StormTech SC-800 +Cap x 35 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 35 Chambers in 7 Rows Cap Storage= 3.4 cf x 2 x 7 rows = 47.9 cf
		3,054 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	51.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	51.00'	12.0" Round Culvert L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 51.00' / 47.00' S= 0.1143 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	53.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	54.50'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

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Discarded OutFlow Max=0.00 cfs @ 1.00 hrs HW=0.00' (Free Discharge)↑ **1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=0.00' (Free Discharge)↑ **2=Culvert** (Controls 0.00 cfs)↑ **3=Orifice/Grate** (Controls 0.00 cfs)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond CB1:**

Inflow Area = 3,316 sf, 100.00% Impervious, Inflow Depth > 5.16" for 25-yr event
Inflow = 0.40 cfs @ 12.08 hrs, Volume= 1,425 cf
Outflow = 0.40 cfs @ 12.08 hrs, Volume= 1,425 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.40 cfs @ 12.08 hrs, Volume= 1,425 cf
Routed to Pond DMH1 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 68.15' @ 12.08 hrs

Flood Elev= 70.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	67.85'	12.0" Round Culvert L= 10.7' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.85' / 67.62' S= 0.0215 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.40 cfs @ 12.08 hrs HW=68.15' TW=67.91' (Dynamic Tailwater)↑ **1=Culvert** (Outlet Controls 0.40 cfs @ 3.00 fps)**Summary for Pond CB2:**

Inflow Area = 2,839 sf, 100.00% Impervious, Inflow Depth > 5.16" for 25-yr event
Inflow = 0.34 cfs @ 12.08 hrs, Volume= 1,220 cf
Outflow = 0.34 cfs @ 12.08 hrs, Volume= 1,220 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.34 cfs @ 12.08 hrs, Volume= 1,220 cf
Routed to Pond DMH1 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 68.12' @ 12.08 hrs

Flood Elev= 70.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	67.85'	12.0" Round RCP_Round 12" L= 5.1' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.85' / 67.62' S= 0.0451 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.34 cfs @ 12.08 hrs HW=68.12' TW=67.91' (Dynamic Tailwater)↑ **1=RCP_Round 12"** (Outlet Controls 0.34 cfs @ 3.02 fps)

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Summary for Pond CB3:

Inflow Area = 4,779 sf, 67.04% Impervious, Inflow Depth > 3.84" for 25-yr event
 Inflow = 0.49 cfs @ 12.09 hrs, Volume= 1,529 cf
 Outflow = 0.49 cfs @ 12.09 hrs, Volume= 1,529 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.49 cfs @ 12.09 hrs, Volume= 1,529 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 55.32' @ 12.09 hrs
 Flood Elev= 57.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	54.90'	12.0" Round RCP_Round 12" L= 11.6' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.90' / 54.58' S= 0.0276 ' S= 0.0276 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.48 cfs @ 12.09 hrs HW=55.32' TW=55.19' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Outlet Controls 0.48 cfs @ 2.29 fps)

Summary for Pond CB4:

Inflow Area = 10,577 sf, 35.20% Impervious, Inflow Depth > 2.68" for 25-yr event
 Inflow = 0.67 cfs @ 12.14 hrs, Volume= 2,364 cf
 Outflow = 0.67 cfs @ 12.14 hrs, Volume= 2,364 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.67 cfs @ 12.14 hrs, Volume= 2,364 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 55.35' @ 12.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.90'	12.0" Round RCP_Round 12" L= 6.4' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.90' / 54.58' S= 0.0500 ' S= 0.0500 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.67 cfs @ 12.14 hrs HW=55.34' TW=55.16' (Dynamic Tailwater)
 ↑1=RCP_Round 12" (Outlet Controls 0.67 cfs @ 2.93 fps)

Summary for Pond DMH1:

Inflow Area = 6,155 sf, 100.00% Impervious, Inflow Depth > 5.16" for 25-yr event
 Inflow = 0.75 cfs @ 12.08 hrs, Volume= 2,646 cf
 Outflow = 0.75 cfs @ 12.08 hrs, Volume= 2,646 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.75 cfs @ 12.08 hrs, Volume= 2,646 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Peak Elev= 67.91' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.52'	12.0" Round RCP_Round 12" L= 202.7' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.52' / 54.58' S= 0.0638 ' S= 0.0638 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.75 cfs @ 12.08 hrs HW=67.91' TW=55.19' (Dynamic Tailwater)

↑1=RCP_Round 12" (Inlet Controls 0.75 cfs @ 2.65 fps)

Summary for Pond DMH2:

Inflow Area = 21,511 sf, 60.82% Impervious, Inflow Depth > 3.65" for 25-yr event
 Inflow = 1.83 cfs @ 12.10 hrs, Volume= 6,538 cf
 Outflow = 1.83 cfs @ 12.10 hrs, Volume= 6,538 cf, Atten= 0%, Lag= 0.0 min
 Primary = 1.83 cfs @ 12.10 hrs, Volume= 6,538 cf
 Routed to Pond IB : DP

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 55.19' @ 12.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.50'	12.0" Round RCP_Round 12" L= 60.8' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.50' / 53.90' S= 0.0099 ' S= 0.0099 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.83 cfs @ 12.10 hrs HW=55.19' TW=53.78' (Dynamic Tailwater)

↑1=RCP_Round 12" (Barrel Controls 1.83 cfs @ 4.44 fps)

Summary for Pond DW1: Drywells

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 5.16" for 25-yr event
 Inflow = 0.34 cfs @ 12.08 hrs, Volume= 1,204 cf
 Outflow = 0.19 cfs @ 12.20 hrs, Volume= 979 cf, Atten= 43%, Lag= 7.0 min
 Discarded = 0.01 cfs @ 9.19 hrs, Volume= 708 cf
 Primary = 0.18 cfs @ 12.20 hrs, Volume= 271 cf
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 3.59' @ 12.20 hrs Surf.Area= 192 sf Storage= 411 cf

Plug-Flow detention time= 175.3 min calculated for 979 cf (81% of inflow)

Center-of-Mass det. time= 101.5 min (847.7 - 746.3)

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Type III 24-hr 25-yr Rainfall=5.40"

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Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
			209 cf x 2.00 = 418 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 9.19 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.18 cfs @ 12.20 hrs HW=3.59' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.18 cfs @ 1.74 fps)**Summary for Pond DW2: Drywells**

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 5.16" for 25-yr event
 Inflow = 0.34 cfs @ 12.08 hrs, Volume= 1,204 cf
 Outflow = 0.19 cfs @ 12.20 hrs, Volume= 979 cf, Atten= 43%, Lag= 7.0 min
 Discarded = 0.01 cfs @ 9.19 hrs, Volume= 708 cf
 Primary = 0.18 cfs @ 12.20 hrs, Volume= 271 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 3.59' @ 12.20 hrs Surf.Area= 192 sf Storage= 411 cf

Plug-Flow detention time= 175.3 min calculated for 979 cf (81% of inflow)

Center-of-Mass det. time= 101.5 min (847.7 - 746.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
			209 cf x 2.00 = 418 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

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Type III 24-hr 25-yr Rainfall=5.40"

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Discarded OutFlow Max=0.01 cfs @ 9.19 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.18 cfs @ 12.20 hrs HW=3.59' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.18 cfs @ 1.74 fps)**Summary for Pond DW3: Drywells**

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 5.16" for 25-yr event
 Inflow = 0.34 cfs @ 12.08 hrs, Volume= 1,204 cf
 Outflow = 0.19 cfs @ 12.20 hrs, Volume= 979 cf, Atten= 43%, Lag= 7.0 min
 Discarded = 0.01 cfs @ 9.19 hrs, Volume= 708 cf
 Primary = 0.18 cfs @ 12.20 hrs, Volume= 271 cf

Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 3.59' @ 12.20 hrs Surf.Area= 192 sf Storage= 411 cf

Plug-Flow detention time= 175.3 min calculated for 979 cf (81% of inflow)

Center-of-Mass det. time= 101.5 min (847.7 - 746.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
209 cf x 2.00 = 418 cf Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 9.19 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.18 cfs @ 12.20 hrs HW=3.59' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.18 cfs @ 1.74 fps)**Summary for Pond IB: DP**

Inflow Area = 31,093 sf, 42.07% Impervious, Inflow Depth > 3.02" for 25-yr event
 Inflow = 2.08 cfs @ 12.11 hrs, Volume= 7,824 cf
 Outflow = 0.40 cfs @ 12.63 hrs, Volume= 6,613 cf, Atten= 81%, Lag= 31.1 min
 Discarded = 0.10 cfs @ 12.63 hrs, Volume= 4,390 cf
 Primary = 0.30 cfs @ 12.63 hrs, Volume= 2,224 cf

Routed to Reach D2 : Large Wetland System

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Type III 24-hr 25-yr Rainfall=5.40"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 54.68' @ 12.63 hrs Surf.Area= 1,850 sf Storage= 3,249 cf

Plug-Flow detention time= 182.7 min calculated for 6,611 cf (84% of inflow)

Center-of-Mass det. time= 115.2 min (919.7 - 804.4)

Volume	Invert	Avail.Storage	Storage Description
#1	52.00'	6,144 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
52.00	458	0	0
53.00	1,113	786	786
54.00	1,530	1,322	2,107
55.00	2,004	1,767	3,874
56.00	2,536	2,270	6,144

Device	Routing	Invert	Outlet Devices
#1	Discarded	52.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	50.00'	12.0" Round RCP_Round 12" L= 42.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 50.00' / 47.00' S= 0.0714 ' / ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#3	Device 2	54.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	55.00'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Discarded OutFlow Max=0.10 cfs @ 12.63 hrs HW=54.68' (Free Discharge)↑ **1=Exfiltration** (Exfiltration Controls 0.10 cfs)**Primary OutFlow** Max=0.30 cfs @ 12.63 hrs HW=54.68' TW=0.00' (Dynamic Tailwater)↑ **2=RCP_Round 12"** (Passes 0.30 cfs of 9.66 cfs potential flow)↑ **3=Orifice/Grate** (Orifice Controls 0.30 cfs @ 3.44 fps)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond IT1: Infil Trench**

Inflow Area = 3,256 sf, 41.92% Impervious, Inflow Depth > 2.77" for 25-yr event
 Inflow = 0.24 cfs @ 12.09 hrs, Volume= 753 cf
 Outflow = 0.24 cfs @ 12.09 hrs, Volume= 753 cf, Atten= 0%, Lag= 0.0 min
 Discarded = 0.00 cfs @ 14.72 hrs, Volume= 0 cf
 Primary = 0.24 cfs @ 12.09 hrs, Volume= 753 cf
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Type III 24-hr 25-yr Rainfall=5.40"

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Peak Elev= 80.50' @ 1.00 hrs Surf.Area= 168 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= (not calculated: outflow precedes inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	80.50'	1,185 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 2,962 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.50	168	0	0
83.00	168	420	420
83.50	10,000	2,542	2,962

Device	Routing	Invert	Outlet Devices
#1	Discarded	80.50'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	59.50'	86.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.00 cfs @ 14.72 hrs HW=80.50' (Free Discharge)↑**1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=80.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Passes 0.00 cfs of 27,476.76 cfs potential flow)**Summary for Pond IT2: Infil Trench**

Inflow Area = 923 sf, 84.83% Impervious, Inflow Depth > 4.05" for 25-yr event
 Inflow = 0.10 cfs @ 12.09 hrs, Volume= 311 cf
 Outflow = 0.01 cfs @ 11.59 hrs, Volume= 311 cf, Atten= 92%, Lag= 0.0 min
 Discarded = 0.01 cfs @ 11.59 hrs, Volume= 311 cf
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 58.68' @ 13.12 hrs Surf.Area= 140 sf Storage= 122 cf

Plug-Flow detention time= 126.0 min calculated for 311 cf (100% of inflow)

Center-of-Mass det. time= 125.8 min (920.3 - 794.5)

Volume	Invert	Avail.Storage	Storage Description
#1	56.50'	1,154 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 2,885 cf Overall x 40.0% Voids

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Type III 24-hr 25-yr Rainfall=5.40"

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
56.50	140	0	0
59.00	140	350	350
59.50	10,000	2,535	2,885

Device	Routing	Invert	Outlet Devices
#1	Discarded	56.50'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	59.50'	72.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.01 cfs @ 11.59 hrs HW=56.53' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=56.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond IT3: Infil Trench**

Inflow Area = 14,390 sf, 16.72% Impervious, Inflow Depth > 2.00" for 25-yr event
 Inflow = 0.65 cfs @ 12.15 hrs, Volume= 2,399 cf
 Outflow = 0.66 cfs @ 12.15 hrs, Volume= 2,179 cf, Atten= 0%, Lag= 0.1 min
 Discarded = 0.03 cfs @ 12.15 hrs, Volume= 679 cf
 Primary = 0.62 cfs @ 12.15 hrs, Volume= 1,500 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 63.52' @ 12.15 hrs Surf.Area= 556 sf Storage= 225 cf

Plug-Flow detention time= 60.1 min calculated for 2,178 cf (91% of inflow)

Center-of-Mass det. time= 15.4 min (871.4 - 856.0)

Volume	Invert	Avail.Storage	Storage Description
#1	61.00'	1,244 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 3,111 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
61.00	222	0	0
63.50	222	555	555
64.00	10,000	2,556	3,111

Device	Routing	Invert	Outlet Devices
#1	Discarded	61.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	63.50'	110.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88

Buonfiglio-10MeyerLn-Prop*Type III 24-hr 25-yr Rainfall=5.40"*

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2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.03 cfs @ 12.15 hrs HW=63.52' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.03 cfs)**Primary OutFlow** Max=0.62 cfs @ 12.15 hrs HW=63.52' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 0.62 cfs @ 0.33 fps)

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Type III 24-hr 100-yr Rainfall=6.50"

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Summary for Subcatchment P1:

Runoff = 7.74 cfs @ 12.21 hrs, Volume= 32,538 cf, Depth> 4.44"
 Routed to Reach C1 : Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

Area (sf)	CN	Description
2,911	61	>75% Grass cover, Good, HSG B
* 49,898	98	Wetland B
35,191	60	Woods, Fair, HSG B
88,000	82	Weighted Average
38,102		43.30% Pervious Area
49,898		56.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	100	0.1100	0.15		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
4.8	280	0.0380	0.97		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
15.8	380	Total			

Summary for Subcatchment P10:

Runoff = 0.48 cfs @ 12.17 hrs, Volume= 1,869 cf, Depth> 2.34"
 Routed to Pond IB : DP

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

Area (sf)	CN	Description
3,125	60	Woods, Fair, HSG B
6,457	61	>75% Grass cover, Good, HSG B
9,582	61	Weighted Average
9,582		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	100	0.1000	0.15		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.10"
0.2	34	0.3300	2.87		Shallow Concentrated Flow, Woods
					Woodland Kv= 5.0 fps
11.7	134	Total			

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Type III 24-hr 100-yr Rainfall=6.50"

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Summary for Subcatchment P11: Lot 3 Dway

Runoff = 0.94 cfs @ 12.15 hrs, Volume= 3,370 cf, Depth> 2.81"
 Routed to Pond IT3 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	2,406	98	Driveway
	7,165	60	Woods, Fair, HSG B
*	222	30	Infil. Trench
	4,597	61	>75% Grass cover, Good, HSG B
	14,390	66	Weighted Average
	11,984		83.28% Pervious Area
	2,406		16.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.8	64	0.0600	0.11		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.10"
0.2	37	0.2000	3.13		Shallow Concentrated Flow, Lawn Short Grass Pasture Kv= 7.0 fps
0.1	16	0.0200	2.87		Shallow Concentrated Flow, Driveway Paved Kv= 20.3 fps
10.1	117	Total			

Summary for Subcatchment P12: Lot 2 Dway

Runoff = 0.12 cfs @ 12.08 hrs, Volume= 393 cf, Depth> 5.10"
 Routed to Pond IT2 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	783	98	Driveway
*	140	30	Infil. Trench
	923	88	Weighted Average
	140		15.17% Pervious Area
	783		84.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

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Type III 24-hr 100-yr Rainfall=6.50"

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Summary for Subcatchment P13: Lot 1 Dway

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 1,006 cf, Depth> 3.71"
 Routed to Pond IT1 : Infil Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	1,365	98	Driveway
*	168	30	Infil. Trench
	1,723	61	>75% Grass cover, Good, HSG B
	3,256	75	Weighted Average
	1,891		58.08% Pervious Area
	1,365		41.92% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P2:

Runoff = 14.49 cfs @ 12.26 hrs, Volume= 64,517 cf, Depth> 3.60"
 Routed to Reach D2 : Large Wetland System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	41,246	98	Wetland B
*	38,423	98	Wetland A
	95,767	60	Woods, Fair, HSG B
	39,869	61	>75% Grass cover, Good, HSG B
	215,305	74	Weighted Average
	135,636		63.00% Pervious Area
	79,669		37.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	100	0.1200	0.16		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
8.0	585	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
18.7	685	Total			

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Type III 24-hr 100-yr Rainfall=6.50"

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Summary for Subcatchment P3:

Runoff = 0.60 cfs @ 12.22 hrs, Volume= 2,527 cf, Depth> 3.50"
 Routed to Reach D3 : 30 Boardman Ln

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

Area (sf)		CN	Description		
*	3,012	98	Wetland B		
	5,660	60	Woods, Fair, HSG B		
	8,672	73	Weighted Average		
	5,660		65.27% Pervious Area		
	3,012		34.73% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0500	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
1.0	69	0.0500	1.12		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
16.1	169	Total			

Summary for Subcatchment P4:

Runoff = 0.94 cfs @ 12.08 hrs, Volume= 3,071 cf, Depth> 5.33"
 Routed to Reach D4 : Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description		
*	5,364	98	Paved parking, HSG B		
	1,551	61	>75% Grass cover, Good, HSG B		
	6,915	90	Weighted Average		
	1,551		22.43% Pervious Area		
	5,364		77.57% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min Tc

Summary for Subcatchment P5:

Runoff = 3.09 cfs @ 12.14 hrs, Volume= 11,024 cf, Depth> 2.34"
 Routed to Reach D1 : 100 & 104 Meyer Rd

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

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Type III 24-hr 100-yr Rainfall=6.50"

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	Area (sf)	CN	Description
*	211	98	Walk, HSG B
	21,956	61	>75% Grass cover, Good, HSG B
*	304	98	Wetland B
	34,018	60	Woods, Fair, HSG B
	56,489	61	Weighted Average
	55,974		99.09% Pervious Area
	515		0.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.8	54	0.0200	1.20		Sheet Flow, Driveway Smooth surfaces n= 0.011 P2= 3.10"
5.3	45	0.0200	0.14		Sheet Flow, Lawn Grass: Short n= 0.150 P2= 3.10"
0.6	78	0.1000	2.21		Shallow Concentrated Flow, Lawn Short Grass Pasture Kv= 7.0 fps
2.4	176	0.0600	1.22		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
9.1	353	Total			

Summary for Subcatchment P6:

Runoff = 0.42 cfs @ 12.08 hrs, Volume= 1,480 cf, Depth> 6.26"
Routed to Pond CB2 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	2,839	98	Paved roads w/curbs, HSG B
	2,839		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P7:

Runoff = 0.49 cfs @ 12.08 hrs, Volume= 1,728 cf, Depth> 6.26"
Routed to Pond CB1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=6.50"

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Type III 24-hr 100-yr Rainfall=6.50"

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	Area (sf)	CN	Description
*	3,316	98	Paved roads w/curbs, HSG B
	3,316		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment P8:

Runoff = 0.90 cfs @ 12.14 hrs, Volume= 3,175 cf, Depth> 3.60"
 Routed to Pond CB4 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	3,723	98	Paved roads w/curbs, HSG B
	6,854	61	>75% Grass cover, Good, HSG B
	10,577	74	Weighted Average
	6,854		64.80% Pervious Area
	3,723		35.20% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.9	61	0.0700	0.11		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.10"
0.6	14	0.5000	0.41		Sheet Flow, Grass Slope Grass: Short n= 0.150 P2= 3.10"
0.3	75	0.0600	4.97		Shallow Concentrated Flow, Roadway Paved Kv= 20.3 fps
9.8	150	Total			

Summary for Subcatchment P9:

Runoff = 0.61 cfs @ 12.09 hrs, Volume= 1,944 cf, Depth> 4.88"
 Routed to Pond CB3 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-yr Rainfall=6.50"

	Area (sf)	CN	Description
*	3,204	98	Paved roads w/curbs, HSG B
	1,575	61	>75% Grass cover, Good, HSG B
	4,779	86	Weighted Average
	1,575		32.96% Pervious Area
	3,204		67.04% Impervious Area

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Type III 24-hr 100-yr Rainfall=6.50"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.5	18	0.0800	0.21		Sheet Flow, Grass
					Grass: Short n= 0.150 P2= 3.10"
0.4	120	0.0700	5.37		Shallow Concentrated Flow, Roadway
					Paved Kv= 20.3 fps
4.1					Direct Entry, Min. Tc
6.0	138	Total			

Summary for Subcatchment R1: Roof

Runoff = 0.41 cfs @ 12.08 hrs, Volume= 1,460 cf, Depth> 6.26"
Routed to Pond DW1 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=6.50"

Area (sf)	CN	Description
* 2,800	98	Roof
2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment R2: Roof

Runoff = 0.41 cfs @ 12.08 hrs, Volume= 1,460 cf, Depth> 6.26"
Routed to Pond DW2 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=6.50"

Area (sf)	CN	Description
* 2,800	98	Roof
2,800		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. Tc

Summary for Subcatchment R3: Roof

Runoff = 0.41 cfs @ 12.08 hrs, Volume= 1,460 cf, Depth> 6.26"
Routed to Pond DW3 : Drywells

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-yr Rainfall=6.50"

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Type III 24-hr 100-yr Rainfall=6.50"

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	Area (sf)	CN	Description
*	2,800	98	Roof
	2,800		100.00% Impervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry, Min. Tc

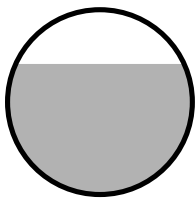
Summary for Reach C1: Culvert

Inflow Area = 88,000 sf, 56.70% Impervious, Inflow Depth > 4.44" for 100-yr event
 Inflow = 7.74 cfs @ 12.21 hrs, Volume= 32,538 cf
 Outflow = 7.74 cfs @ 12.21 hrs, Volume= 32,537 cf, Atten= 0%, Lag= 0.0 min
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Max. Velocity= 13.07 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 5.16 fps, Avg. Travel Time= 0.1 min

Peak Storage= 21 cf @ 12.21 hrs
 Average Depth at Peak Storage= 0.71', Surface Width= 0.91'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 9.15 cfs

12.0" Round Pipe
 n= 0.011 Concrete pipe, straight & clean
 Length= 36.0' Slope= 0.0472 '/
 Inlet Invert= 68.30', Outlet Invert= 66.60'

**Summary for Reach D1: 100 & 104 Meyer Rd**

Inflow Area = 150,545 sf, 36.25% Impervious, Inflow Depth > 3.59" for 100-yr event
 Inflow = 10.85 cfs @ 12.18 hrs, Volume= 45,035 cf
 Outflow = 10.85 cfs @ 12.18 hrs, Volume= 45,035 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D2: Large Wetland System

Inflow Area = 267,311 sf, 37.99% Impervious, Inflow Depth > 3.23" for 100-yr event
 Inflow = 15.87 cfs @ 12.24 hrs, Volume= 71,891 cf
 Outflow = 15.87 cfs @ 12.24 hrs, Volume= 71,891 cf, Atten= 0%, Lag= 0.0 min

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D3: 30 Boardman Ln

Inflow Area = 8,672 sf, 34.73% Impervious, Inflow Depth > 3.50" for 100-yr event
 Inflow = 0.60 cfs @ 12.22 hrs, Volume= 2,527 cf
 Outflow = 0.60 cfs @ 12.22 hrs, Volume= 2,527 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Reach D4: Meyer Rd

Inflow Area = 6,915 sf, 77.57% Impervious, Inflow Depth > 5.33" for 100-yr event
 Inflow = 0.94 cfs @ 12.08 hrs, Volume= 3,071 cf
 Outflow = 0.94 cfs @ 12.08 hrs, Volume= 3,071 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Summary for Pond 3P: DP

Volume	Invert	Avail.Storage	Storage Description
#1A	51.00'	1,235 cf	33.25'W x 39.35'L x 3.75'H Field A 4,906 cf Overall - 1,819 cf Embedded = 3,088 cf x 40.0% Voids
#2A	51.50'	1,819 cf	ADS_StormTech SC-800 +Cap x 35 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 35 Chambers in 7 Rows Cap Storage= 3.4 cf x 2 x 7 rows = 47.9 cf
		3,054 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	51.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	51.00'	12.0" Round Culvert L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 51.00' / 47.00' S= 0.1143 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	53.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	54.50'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

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Type III 24-hr 100-yr Rainfall=6.50"

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Discarded OutFlow Max=0.00 cfs @ 1.00 hrs HW=0.00' (Free Discharge)↑ **1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=0.00' (Free Discharge)↑ **2=Culvert** (Controls 0.00 cfs)↑ **3=Orifice/Grate** (Controls 0.00 cfs)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond CB1:**

Inflow Area = 3,316 sf, 100.00% Impervious, Inflow Depth > 6.26" for 100-yr event
 Inflow = 0.49 cfs @ 12.08 hrs, Volume= 1,728 cf
 Outflow = 0.49 cfs @ 12.08 hrs, Volume= 1,728 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.49 cfs @ 12.08 hrs, Volume= 1,728 cf
 Routed to Pond DMH1 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 68.19' @ 12.08 hrs

Flood Elev= 70.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	67.85'	12.0" Round Culvert L= 10.7' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.85' / 67.62' S= 0.0215 ' / ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.48 cfs @ 12.08 hrs HW=68.19' TW=67.95' (Dynamic Tailwater)↑ **1=Culvert** (Outlet Controls 0.48 cfs @ 3.05 fps)**Summary for Pond CB2:**

Inflow Area = 2,839 sf, 100.00% Impervious, Inflow Depth > 6.26" for 100-yr event
 Inflow = 0.42 cfs @ 12.08 hrs, Volume= 1,480 cf
 Outflow = 0.42 cfs @ 12.08 hrs, Volume= 1,480 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.42 cfs @ 12.08 hrs, Volume= 1,480 cf
 Routed to Pond DMH1 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 68.16' @ 12.08 hrs

Flood Elev= 70.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	67.85'	12.0" Round RCP_Round 12" L= 5.1' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.85' / 67.62' S= 0.0451 ' / ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.41 cfs @ 12.08 hrs HW=68.16' TW=67.95' (Dynamic Tailwater)↑ **1=RCP_Round 12"** (Outlet Controls 0.41 cfs @ 3.03 fps)

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Type III 24-hr 100-yr Rainfall=6.50"

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Summary for Pond CB3:

Inflow Area = 4,779 sf, 67.04% Impervious, Inflow Depth > 4.88" for 100-yr event
 Inflow = 0.61 cfs @ 12.09 hrs, Volume= 1,944 cf
 Outflow = 0.61 cfs @ 12.09 hrs, Volume= 1,944 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.61 cfs @ 12.09 hrs, Volume= 1,944 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 55.42' @ 12.09 hrs

Flood Elev= 57.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	54.90'	12.0" Round RCP_Round 12" L= 11.6' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.90' / 54.58' S= 0.0276 ' S= 0.0276 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.61 cfs @ 12.09 hrs HW=55.41' TW=55.30' (Dynamic Tailwater)

↑1=RCP_Round 12" (Outlet Controls 0.61 cfs @ 2.18 fps)

Summary for Pond CB4:

Inflow Area = 10,577 sf, 35.20% Impervious, Inflow Depth > 3.60" for 100-yr event
 Inflow = 0.90 cfs @ 12.14 hrs, Volume= 3,175 cf
 Outflow = 0.90 cfs @ 12.14 hrs, Volume= 3,175 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.90 cfs @ 12.14 hrs, Volume= 3,175 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 55.47' @ 12.11 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.90'	12.0" Round RCP_Round 12" L= 6.4' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.90' / 54.58' S= 0.0500 ' S= 0.0500 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.90 cfs @ 12.14 hrs HW=55.45' TW=55.27' (Dynamic Tailwater)

↑1=RCP_Round 12" (Outlet Controls 0.90 cfs @ 2.94 fps)

Summary for Pond DMH1:

Inflow Area = 6,155 sf, 100.00% Impervious, Inflow Depth > 6.26" for 100-yr event
 Inflow = 0.90 cfs @ 12.08 hrs, Volume= 3,208 cf
 Outflow = 0.90 cfs @ 12.08 hrs, Volume= 3,208 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.90 cfs @ 12.08 hrs, Volume= 3,208 cf
 Routed to Pond DMH2 :

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

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Type III 24-hr 100-yr Rainfall=6.50"

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Peak Elev= 67.95' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.52'	12.0" Round RCP_Round 12" L= 202.7' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 67.52' / 54.58' S= 0.0638 ' S= 0.0638 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.90 cfs @ 12.08 hrs HW=67.95' TW=55.30' (Dynamic Tailwater)

↑1=RCP_Round 12" (Inlet Controls 0.90 cfs @ 2.79 fps)

Summary for Pond DMH2:

Inflow Area = 21,511 sf, 60.82% Impervious, Inflow Depth > 4.65" for 100-yr event
Inflow = 2.32 cfs @ 12.10 hrs, Volume= 8,328 cf
Outflow = 2.32 cfs @ 12.10 hrs, Volume= 8,328 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.32 cfs @ 12.10 hrs, Volume= 8,328 cf
Routed to Pond IB : DP

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 55.31' @ 12.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.50'	12.0" Round RCP_Round 12" L= 60.8' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 54.50' / 53.90' S= 0.0099 ' S= 0.0099 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=2.32 cfs @ 12.10 hrs HW=55.31' TW=54.24' (Dynamic Tailwater)

↑1=RCP_Round 12" (Barrel Controls 2.32 cfs @ 4.66 fps)

Summary for Pond DW1: Drywells

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 6.26" for 100-yr event
Inflow = 0.41 cfs @ 12.08 hrs, Volume= 1,460 cf
Outflow = 0.40 cfs @ 12.10 hrs, Volume= 1,205 cf, Atten= 3%, Lag= 1.0 min
Discarded = 0.01 cfs @ 8.66 hrs, Volume= 738 cf
Primary = 0.39 cfs @ 12.10 hrs, Volume= 467 cf
Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 3.75' @ 12.10 hrs Surf.Area= 192 sf Storage= 416 cf

Plug-Flow detention time= 148.0 min calculated for 1,205 cf (83% of inflow)

Center-of-Mass det. time= 76.7 min (820.3 - 743.6)

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Type III 24-hr 100-yr Rainfall=6.50"

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Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
209 cf x 2.00 = 418 cf			Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 8.66 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.39 cfs @ 12.10 hrs HW=3.75' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.39 cfs @ 2.20 fps)**Summary for Pond DW2: Drywells**

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 6.26" for 100-yr event
 Inflow = 0.41 cfs @ 12.08 hrs, Volume= 1,460 cf
 Outflow = 0.40 cfs @ 12.10 hrs, Volume= 1,205 cf, Atten= 3%, Lag= 1.0 min
 Discarded = 0.01 cfs @ 8.66 hrs, Volume= 738 cf
 Primary = 0.39 cfs @ 12.10 hrs, Volume= 467 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 3.75' @ 12.10 hrs Surf.Area= 192 sf Storage= 416 cf

Plug-Flow detention time= 148.0 min calculated for 1,205 cf (83% of inflow)

Center-of-Mass det. time= 76.7 min (820.3 - 743.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
209 cf x 2.00 = 418 cf			Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

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Type III 24-hr 100-yr Rainfall=6.50"

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Discarded OutFlow Max=0.01 cfs @ 8.66 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.39 cfs @ 12.10 hrs HW=3.75' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.39 cfs @ 2.20 fps)**Summary for Pond DW3: Drywells**

Inflow Area = 2,800 sf, 100.00% Impervious, Inflow Depth > 6.26" for 100-yr event
 Inflow = 0.41 cfs @ 12.08 hrs, Volume= 1,460 cf
 Outflow = 0.40 cfs @ 12.10 hrs, Volume= 1,205 cf, Atten= 3%, Lag= 1.0 min
 Discarded = 0.01 cfs @ 8.66 hrs, Volume= 738 cf
 Primary = 0.39 cfs @ 12.10 hrs, Volume= 467 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 3.75' @ 12.10 hrs Surf.Area= 192 sf Storage= 416 cf

Plug-Flow detention time= 148.0 min calculated for 1,205 cf (83% of inflow)

Center-of-Mass det. time= 76.7 min (820.3 - 743.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	80 cf	7.67'W x 12.50'L x 3.83'H Field A 367 cf Overall - 166 cf Embedded = 201 cf x 40.0% Voids
#2A	1.00'	129 cf	Shea Dry Well 1000gal Inside #1 Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.80 sf x 10.50'L = 165.9 cf
209 cf x 2.00 = 418 cf Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	3.33'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 8.66 hrs HW=0.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.01 cfs)**Primary OutFlow** Max=0.39 cfs @ 12.10 hrs HW=3.75' TW=0.00' (Dynamic Tailwater)↑**2=Orifice/Grate** (Orifice Controls 0.39 cfs @ 2.20 fps)**Summary for Pond IB: DP**

Inflow Area = 31,093 sf, 42.07% Impervious, Inflow Depth > 3.94" for 100-yr event
 Inflow = 2.72 cfs @ 12.11 hrs, Volume= 10,197 cf
 Outflow = 0.89 cfs @ 12.49 hrs, Volume= 8,747 cf, Atten= 67%, Lag= 22.7 min
 Discarded = 0.11 cfs @ 12.49 hrs, Volume= 4,709 cf
 Primary = 0.77 cfs @ 12.49 hrs, Volume= 4,038 cf
 Routed to Reach D2 : Large Wetland System

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Type III 24-hr 100-yr Rainfall=6.50"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 55.10' @ 12.49 hrs Surf.Area= 2,057 sf Storage= 4,075 cf

Plug-Flow detention time= 156.9 min calculated for 8,747 cf (86% of inflow)

Center-of-Mass det. time= 93.5 min (894.0 - 800.5)

Volume	Invert	Avail.Storage	Storage Description
#1	52.00'	6,144 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
52.00	458	0	0
53.00	1,113	786	786
54.00	1,530	1,322	2,107
55.00	2,004	1,767	3,874
56.00	2,536	2,270	6,144

Device	Routing	Invert	Outlet Devices
#1	Discarded	52.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	50.00'	12.0" Round RCP_Round 12" L= 42.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= 50.00' / 47.00' S= 0.0714 ' / ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#3	Device 2	54.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	55.00'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Discarded OutFlow Max=0.11 cfs @ 12.49 hrs HW=55.10' (Free Discharge)↑ **1=Exfiltration** (Exfiltration Controls 0.11 cfs)**Primary OutFlow** Max=0.77 cfs @ 12.49 hrs HW=55.10' TW=0.00' (Dynamic Tailwater)↑ **2=RCP_Round 12"** (Passes 0.41 cfs of 10.14 cfs potential flow)↑ **3=Orifice/Grate** (Orifice Controls 0.41 cfs @ 4.65 fps)↑ **4=Broad-Crested Rectangular Weir** (Weir Controls 0.37 cfs @ 0.74 fps)**Summary for Pond IT1: Infil Trench**

Inflow Area = 3,256 sf, 41.92% Impervious, Inflow Depth > 3.71" for 100-yr event
 Inflow = 0.33 cfs @ 12.09 hrs, Volume= 1,006 cf
 Outflow = 0.33 cfs @ 12.09 hrs, Volume= 1,006 cf, Atten= 0%, Lag= 0.0 min
 Discarded = 0.00 cfs @ 15.53 hrs, Volume= 0 cf
 Primary = 0.33 cfs @ 12.09 hrs, Volume= 1,006 cf
 Routed to Reach D1 : 100 & 104 Meyer Rd

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Buonfiglio-10MeyerLn-Prop

Type III 24-hr 100-yr Rainfall=6.50"

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Peak Elev= 80.50' @ 1.00 hrs Surf.Area= 168 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 0.0 min (822.2 - 822.2)

Volume	Invert	Avail.Storage	Storage Description
#1	80.50'	1,185 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 2,962 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.50	168	0	0
83.00	168	420	420
83.50	10,000	2,542	2,962

Device	Routing	Invert	Outlet Devices
#1	Discarded	80.50'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	59.50'	86.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.00 cfs @ 15.53 hrs HW=80.50' (Free Discharge)↑**1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=80.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Passes 0.00 cfs of 27,476.76 cfs potential flow)**Summary for Pond IT2: Infil Trench**

Inflow Area = 923 sf, 84.83% Impervious, Inflow Depth > 5.10" for 100-yr event
 Inflow = 0.12 cfs @ 12.08 hrs, Volume= 393 cf
 Outflow = 0.03 cfs @ 12.45 hrs, Volume= 392 cf, Atten= 74%, Lag= 22.2 min
 Discarded = 0.03 cfs @ 12.45 hrs, Volume= 392 cf
 Primary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 59.02' @ 12.45 hrs Surf.Area= 562 sf Storage= 143 cf

Plug-Flow detention time= 139.8 min calculated for 392 cf (100% of inflow)

Center-of-Mass det. time= 139.7 min (927.8 - 788.2)

Volume	Invert	Avail.Storage	Storage Description
#1	56.50'	1,154 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 2,885 cf Overall x 40.0% Voids

Buonfiglio-10MeyerLn-Prop

Type III 24-hr 100-yr Rainfall=6.50"

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
56.50	140	0	0
59.00	140	350	350
59.50	10,000	2,535	2,885

Device	Routing	Invert	Outlet Devices
#1	Discarded	56.50'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	59.50'	72.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.03 cfs @ 12.45 hrs HW=59.02' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.03 cfs)**Primary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=56.50' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond IT3: Infil Trench**

Inflow Area = 14,390 sf, 16.72% Impervious, Inflow Depth > 2.81" for 100-yr event
 Inflow = 0.94 cfs @ 12.15 hrs, Volume= 3,370 cf
 Outflow = 0.94 cfs @ 12.15 hrs, Volume= 3,148 cf, Atten= 0%, Lag= 0.0 min
 Discarded = 0.04 cfs @ 12.15 hrs, Volume= 747 cf
 Primary = 0.90 cfs @ 12.15 hrs, Volume= 2,401 cf
 Routed to Reach D2 : Large Wetland System

Routing by Dyn-Stor-Ind method, Time Span= 1.00-24.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 63.52' @ 12.15 hrs Surf.Area= 649 sf Storage= 226 cf

Plug-Flow detention time= 44.7 min calculated for 3,148 cf (93% of inflow)

Center-of-Mass det. time= 10.7 min (856.6 - 845.9)

Volume	Invert	Avail.Storage	Storage Description
#1	61.00'	1,244 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 3,111 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
61.00	222	0	0
63.50	222	555	555
64.00	10,000	2,556	3,111

Device	Routing	Invert	Outlet Devices
#1	Discarded	61.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	63.50'	110.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88

Buonfiglio-10MeyerLn-Prop*Type III 24-hr 100-yr Rainfall=6.50"*

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2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.04 cfs @ 12.15 hrs HW=63.52' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.04 cfs)**Primary OutFlow** Max=0.90 cfs @ 12.15 hrs HW=63.52' TW=0.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 0.90 cfs @ 0.38 fps)